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SECTION TITLE	UNIT		SECTION NO.	REFERENCE NO.	SECTION TITLE		DRO DOC
VAL OF STRUCTURES AND OBSTRUCTIONS	L.S.	LS.	705	22	P.C.C. CURB (TYPE I)		PROPOS
VAL OF STRUCTURES AND OBSTRUCTIONS	LBS	43.140 75,080	708	23	P.C.C. SIDEWALK, 4"	L.F.	36
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CLASS B	C.Y.		S.P.	30	SAW CUTTING (CONC.)	SYIN.	(~~)··
IRS, CLASS A	SYIN.	2184-10-	S.P.	31	BUTT JOINT	L.F.	
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EINFORCEMENT (EPOXY COATED)	LBS.	1971		PE	CLEANING AND PAINTING EXIST. STEEL STRUCTURE	<i>L,S</i> ,	45
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			S.R.	35	MAINTENANCE OF TRAFFIC	L.S,	L.S.
STRUCTURES (REPAIR WEB STIFFENERS)		The has not for 5 and	S.P.	36	INITIAL EXPENSE	L.S.	L.S
STRUCTURES (MISCELLANEOUS REPAIRS)	LBS.	6,000-	S.P.		STEEL GRID FLOORING AND SIDEWALK GRATING	2.0.	
BRIDGE RAILING (ALUMINUM)	<i>L.F.</i>	an 42-5 -			BASCULE SPAN SIDEWALK	S.F.	
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DIVISION ADMINISTRATOR

1. EXISTING DRAWINGS

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-This structure was built in 1929-1930

-The bascule span was refloored and new wearing surface placed on the approach spans in 1948-1950.

-The concrete slab was replaced in 1966.

-Contract and shop drawings defining the previous construction are on file in the Department of Transportation, Dover, Delaware and are available for reference, however the Department accepts no responsibility for the completeness or accuracy of the data and information contained in these drawings. It will be the Contractor responsibility to verify this information.

2. <u>SPICIFICATIONS</u>

Design: Standard Specifications for Highway Bridges AASHTO 1983 and interim specifications to date.

Construction: Delaware Dept. of Highways and Transportation Standard Specification 1974 and Delaware Department of Transportation Supplemental Specification, 1980.

LIVE LOAD: HS 20-44

3. <u>CONCRETE</u>

Concrete Sidewalk and Slab on Approach Span shall be Class A and shall have a minimun 28 day compressive strength (f'c) of 4,500psi. All other concrete shall be Class B and shall have a minimum 28 day compressive strength (f'c) of 3000psi. Minimum cover over reinforcing bars shall be 2" except shown otherwise. All exposed edges of concrete shall be chamfered 3/4 in.

REINFORCING BARS 4.

VISIONS

Bar reinforcement shall conform to ASTM A615, Grade 60. If field splices are required, the minimum length of splice shall be 40 times the bar diameter.

5. <u>STEEL STRUCTURES</u>

- a) Unless otherwise noted all structural steel shall be ASTM A572 and painted.
- b) All welding shall be in accordance with the current Structural Welding Code of the American Welding Society and Standard Specification for Welding of Structural Steel Highway Bridges published by AASHTO.
- c) All damaged rivets (50% or greater loss of head) should be replaced by the Contractor with equal size friction - type ASTM A 325 bolts. Where a group of rivets is being replaced, one rivet at a time shall be removed and immediately replaced with a high strength bolt before the next rivet is removed. All new bolts specified in these plans are 7/8° u.e. A 320 with open holes 15/16 Dia. unlass otherwise noted. All bolts, nuts and washers shall conform to or be compatible with ASTM A 325, and mechanically galvanized in accordance with ASTM 8 454.

6. <u>Datum</u>

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elevations. It verify the el	be the Contracto	responsibility to
drawings.	nes persue the b	aration or shop

7. EXISTING DIMENSIONS

Unless otherwise noted, dimensions shown for the existing structure are taken from the iginal contract place. The Contractor is responsible for the infying the accuracy of this information and shall noti, the department of all deviation, if any, before work begins.

- 8. <u>REPAIR SEQUENCE</u>
 - a) A suggested repair sequence is shown throughout these plans. The Contractor may propose an alternative sequence.
 - b) The Contractor shall submit for approval his proposed repair sequence before beginning work. The proposed sequence shall take into account the effects or damage on the existing structure and nembers not to be remaining.
 - c) It is planned to detour roadway (raffic away from the structure during repair work.
 - d) During struction period raising the span will be necessary of the replacement of the rest pier. The Contractor shall make his own arrangements for these openings, see Special Provisions.

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b) The existing aluminum railing located in front of the operator's house shall be salvaged. Payment for this work shall be incidental to the contract.

c) Payment for raising bascule span and jacking of South Approach span during reconstruction of the rest pier shall be incidental to Section 606 - Steel Structures

d) The existing NGS Control located on top of parapet near the operator's house (see sheet #3) shall not be disturbed during construction.

e) All drains on the bridge deck and expansion joints shall be thoroughly cleaned. Payment to be included in various bid items.

10. MAINTENANCE OF TRAFFIC

The contractor shall perform all work in a manner that will ensure the least practicable obstruction to traffic consistent with safety and shall comply with the manual entitled, "Delaware Traffic Controls for Street and Highway Construction and Maintenance Operations" (as revised January

e) Suggested Repair Sequence:

- Detour traffic.
- 2) Follow steps indicated in Bascule Span Drawings to remove and replace members of the superstructure.
- 3) Remove and place new piles of the north fender system after removal of all stringers from the span 4.
- 4) Remove concrete sidewalk and slab indicated in replacement of concrete floor South Approach Slab sheet #21.
- 5) Open bascule span.
- 6) Follow steps indicated in the Rest Pier Brawing (Sheet #22).
- Remove hydraulic jacks and reset main girder.
- 8) Follow steps indicated in floorbeam F/O Drawings (Sheet #18) to remove members of the superstructure.
- 9) Remove temporary floorbeam support from the South Approach.
- 10) Remove and place new piles of the south fender system.
- 11) Close bascule span.
- 12) Place new concrete sidewalk and slab at the South Approach Slab.
- 13) Remove al railing (concrete and steel) and replace with new aluminum railing. Replace pedestrian sidewalk on bascule span
- 14) Concrete sidewalks and concrete deck repair.
- 15. Reopen to highway traffic.
- 16) Complete finish pathoning and clean up site.

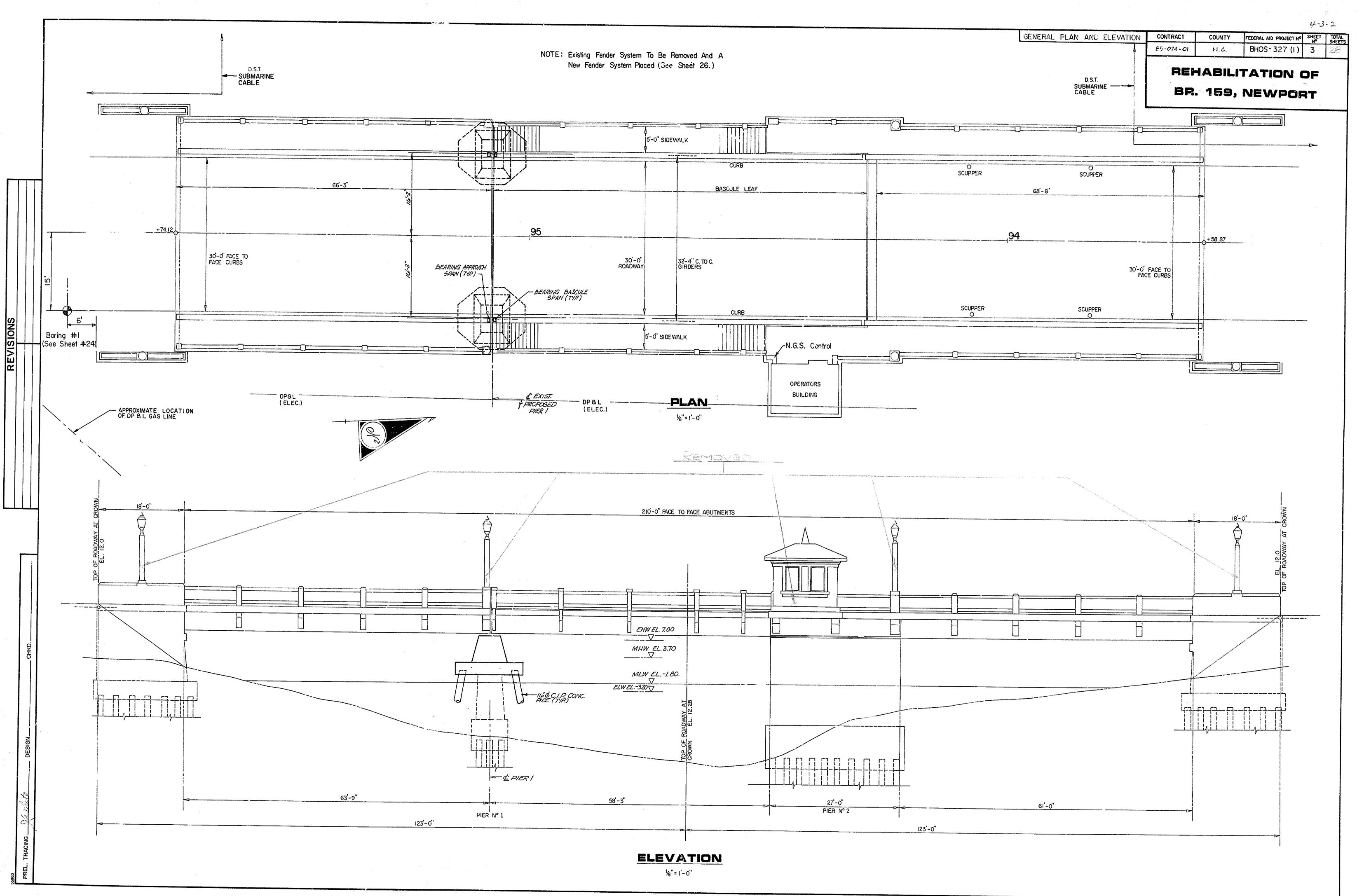
a) The existing operator's house shall be sealed shut by nailing plywood sheets to all openings. Payment for this will be incidental to the contract.

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and Marine Marine	6.	Floorbeam F/2 and B					
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	8.	Brackets FB/5 and B					
1799-1200 - 1 0, 10, 17	9.	Floorbeam F/5 and S					
	10.	Bascule Span Main Support (1)					
;	11.	Bascule Span Main (Support (3)	Girder Details	and Repair	s and Sidex		
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	23.	Bearing Details	······································				
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	2 6.	Rebalance of Bescule	Span				
	27.	Fender System					
	27.	Striping Plan					
	28.	Standard Sheet Cl					

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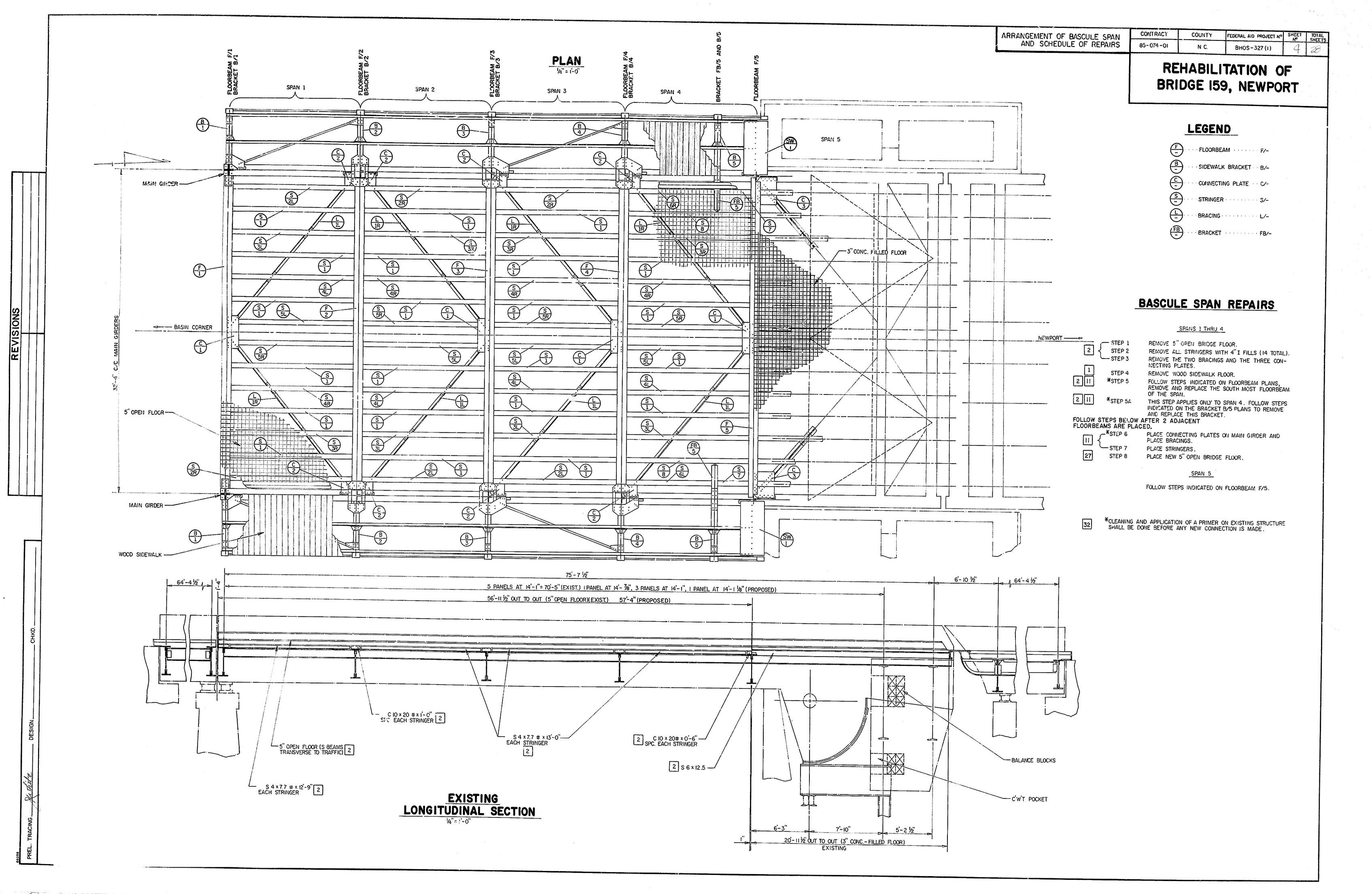
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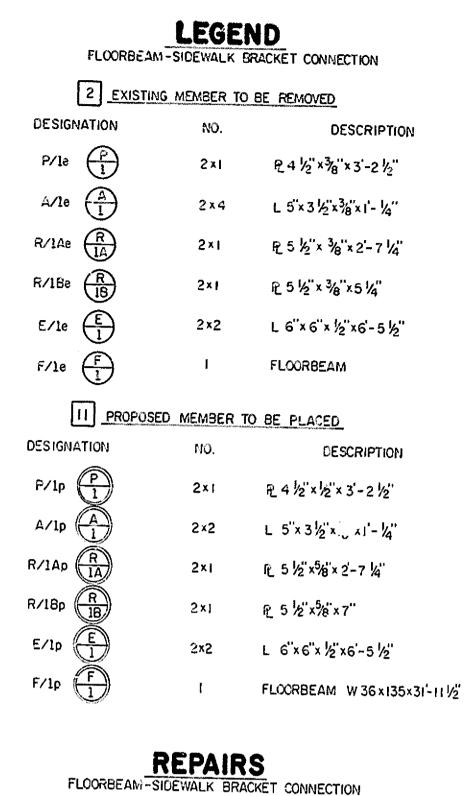


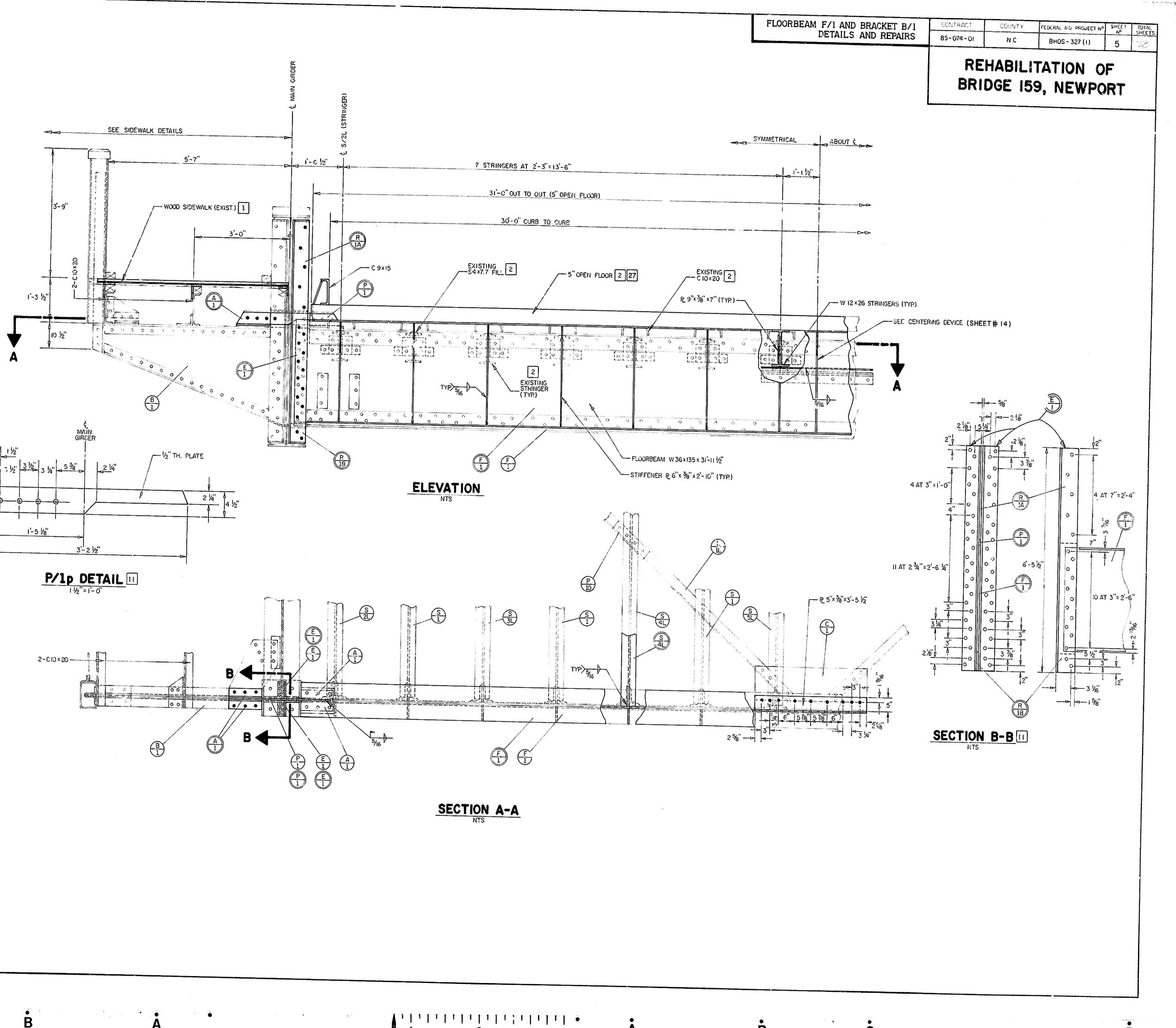
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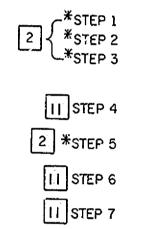
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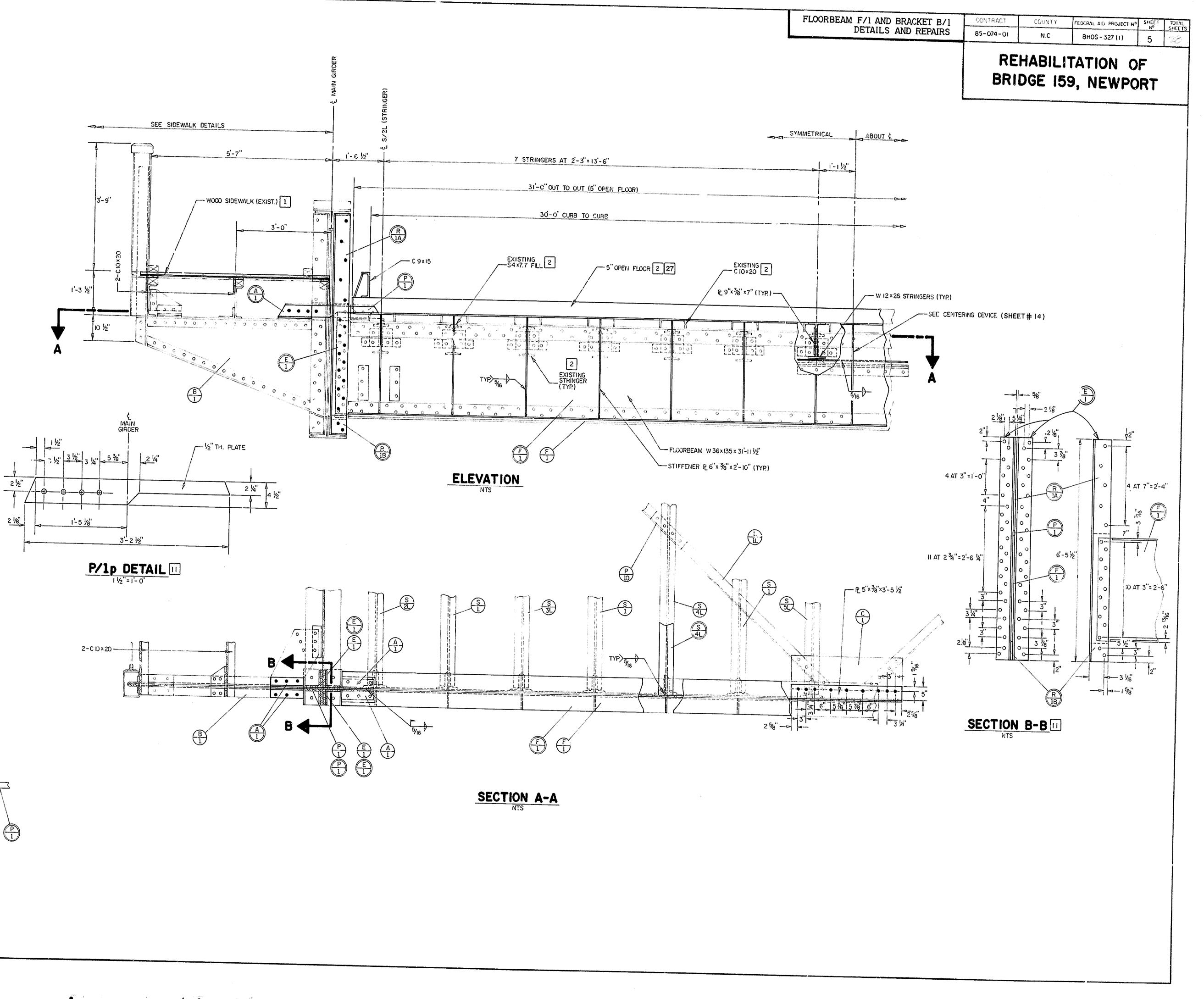


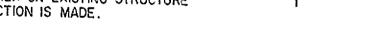




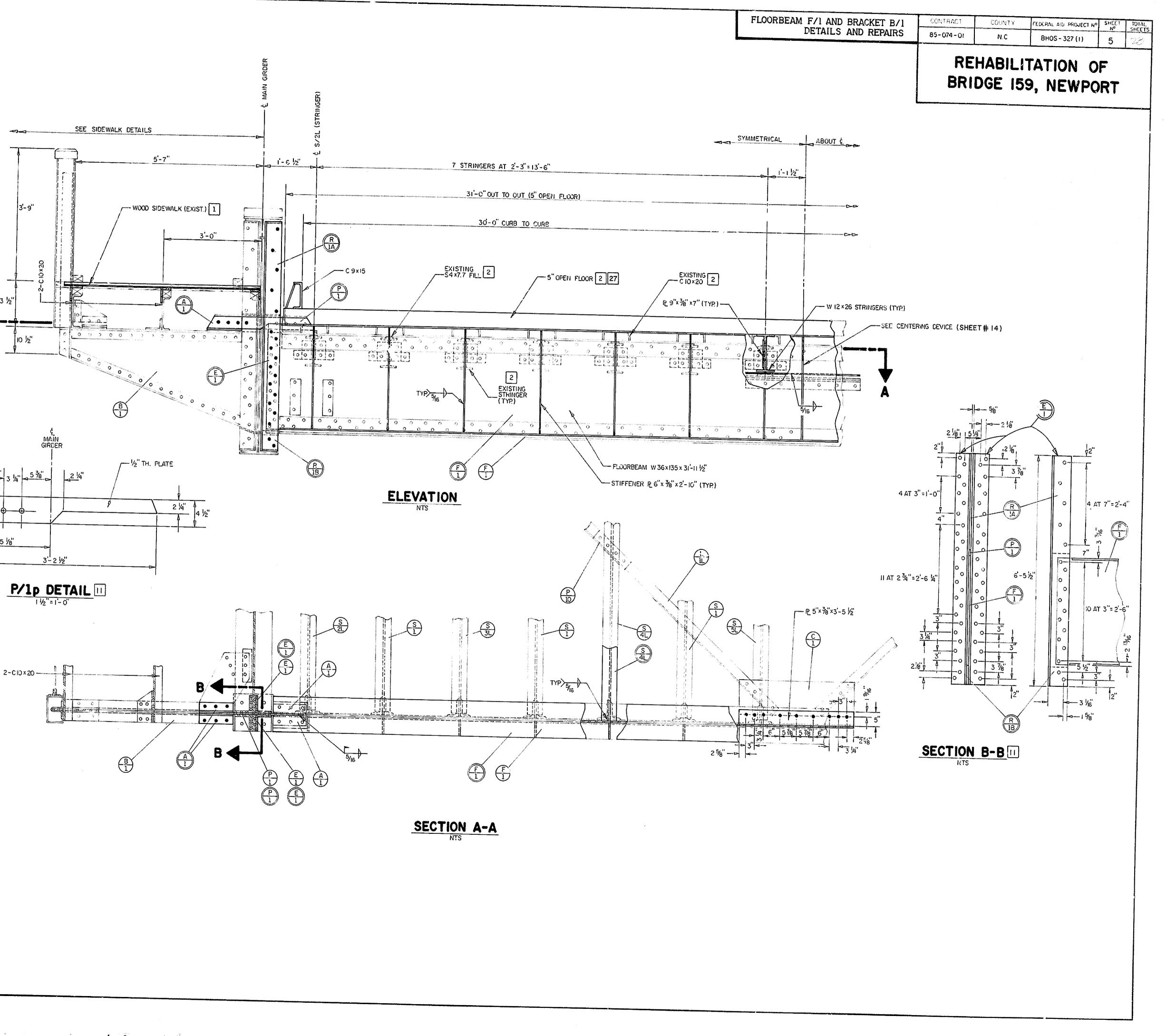
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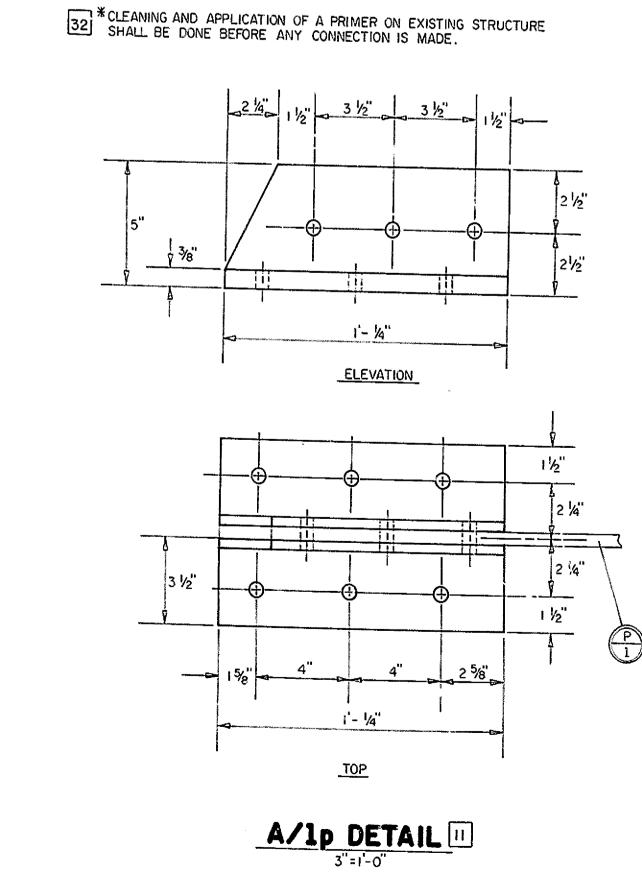
REEAM-SIDEWALK BRACKET CONNECTION
REMOVE MEMBERS A/le TO RELEASE P/le.
REMOVE P/1e THROUGH SLOT IN GIRDER.
REMOVE ONE E/10 FROM EACH MAIN GIRDER. REMOVE FILLS R/1A0 AND R/1B0. REMOVE FLOORBEAM F/10.
REPLACE EACH E/10 REMOVED WITH E/1p IN EACH MAIN GIRDER.
REMOVE REMAINING E/le IN EACH MAIN GIRDER
PLACE FILLS R/1Ap AND R/1Bp. PLACE FLOOR- BEAM F/1p AND PLACE E/1p.
PLACE MEMBERS P/1e AND A/1e.





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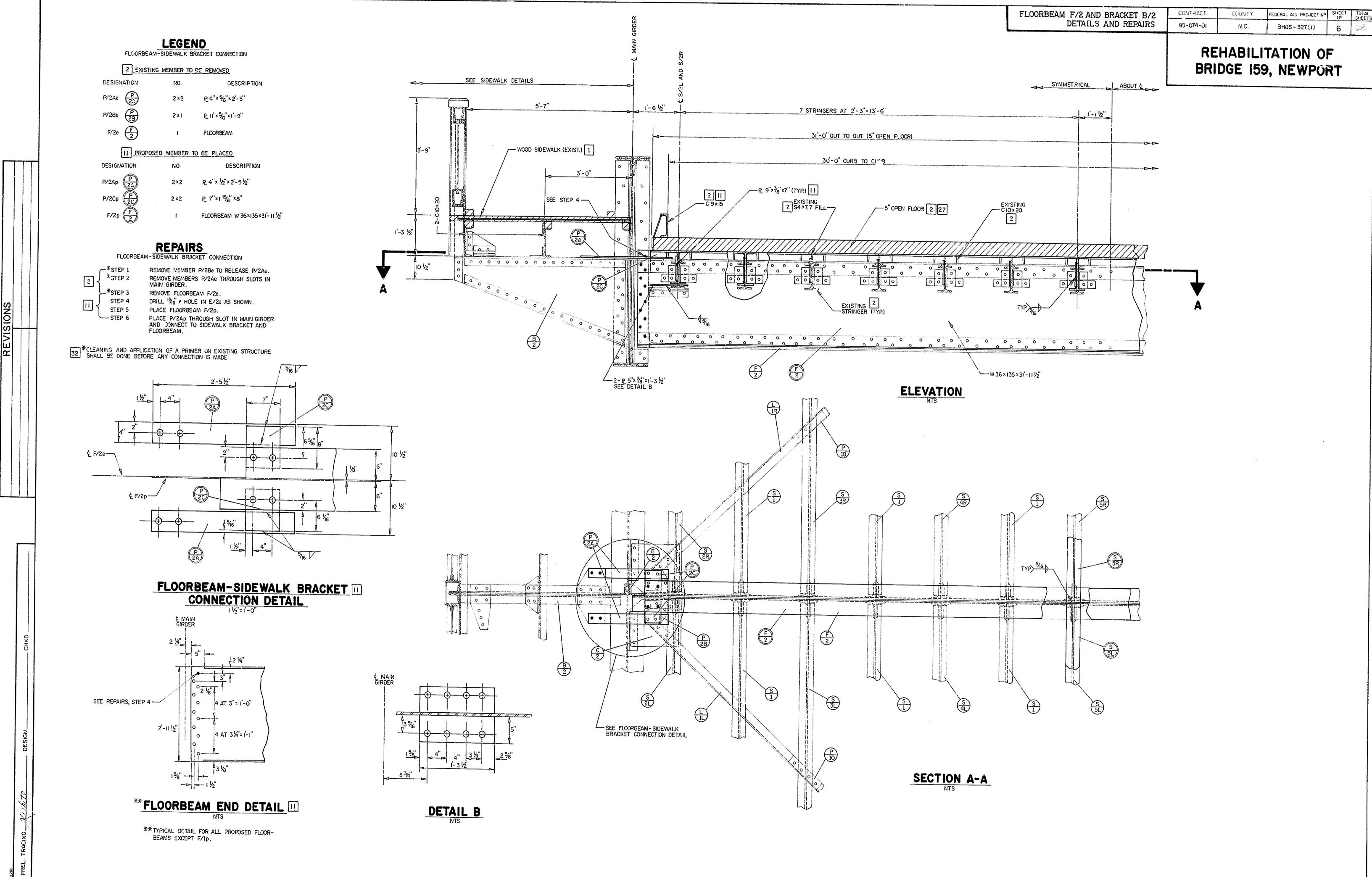


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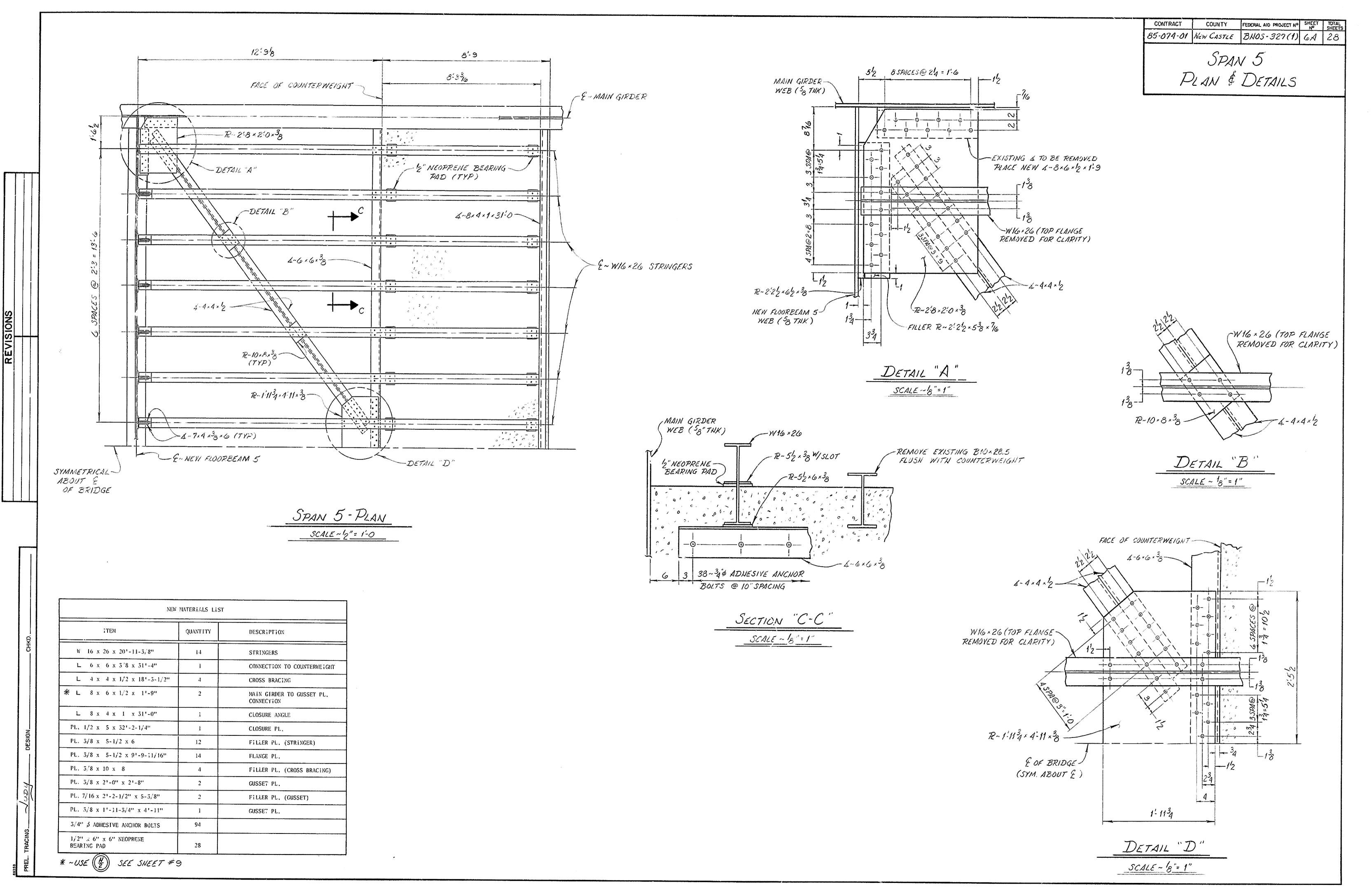
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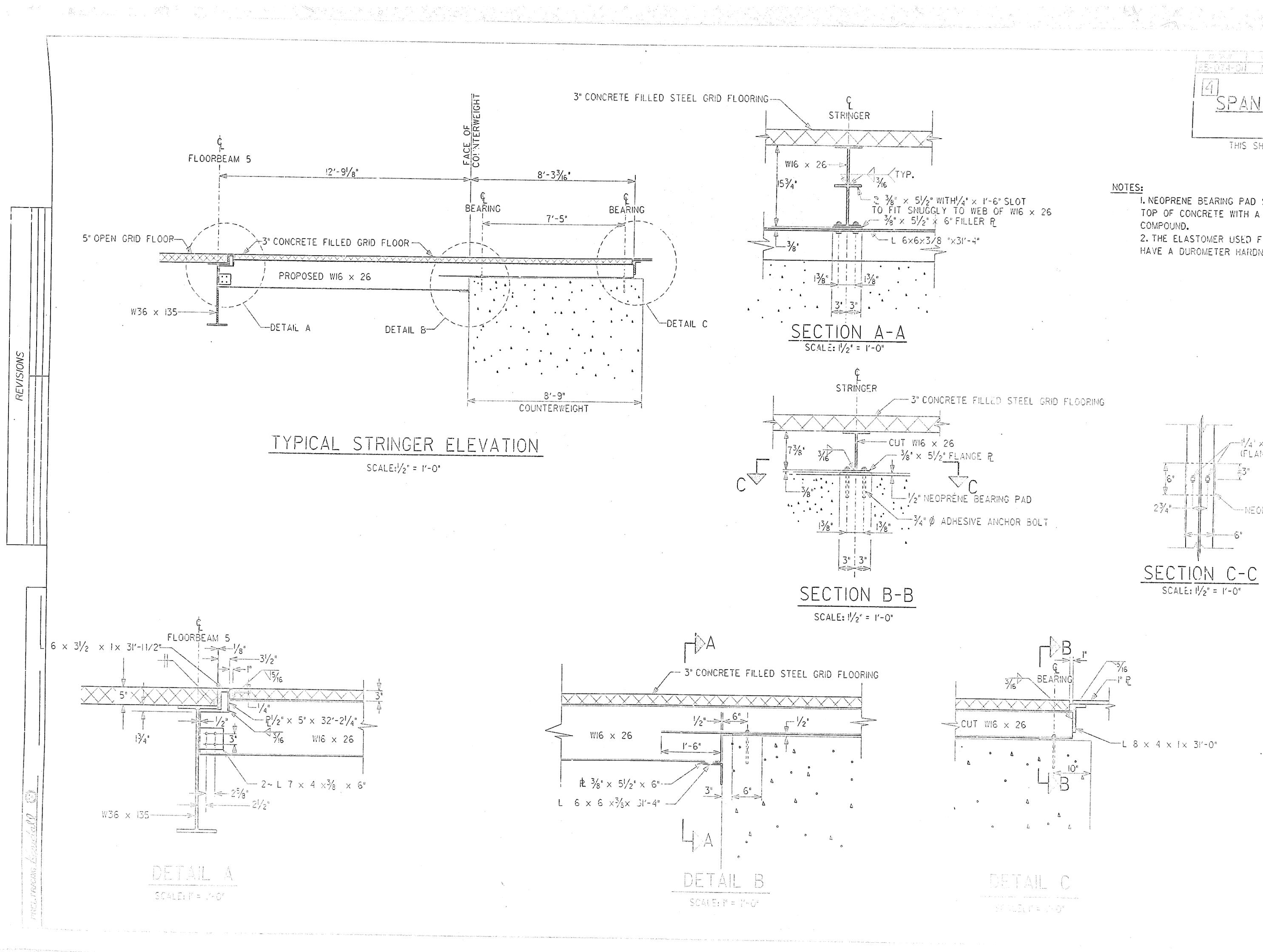
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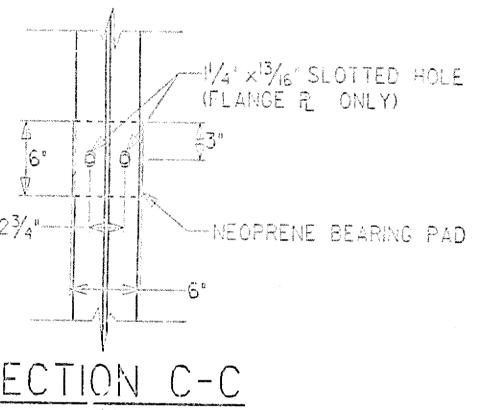
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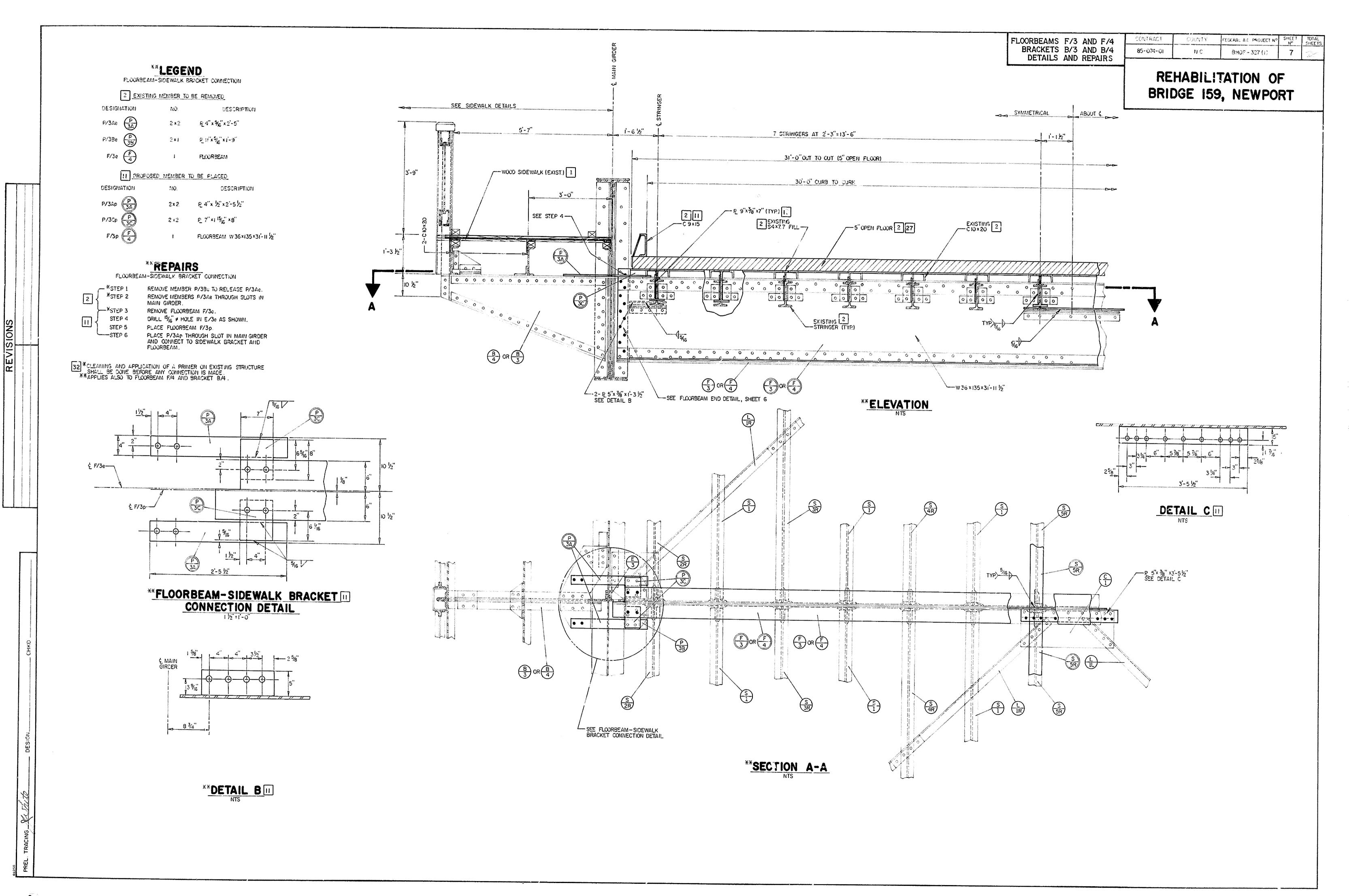
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THIS SHEET IS A REVISION

I. NEOPRENE BEARING PAD SHALL BE BONDED TO THE TOP OF CONCRETE WITH A SUITABLE EPOXY BONDING

2. THE ELASTOMER USED FOR THE BEAFING PAD SHALL HAVE A DUROMETER HARDNESS OF 50 TO 60.





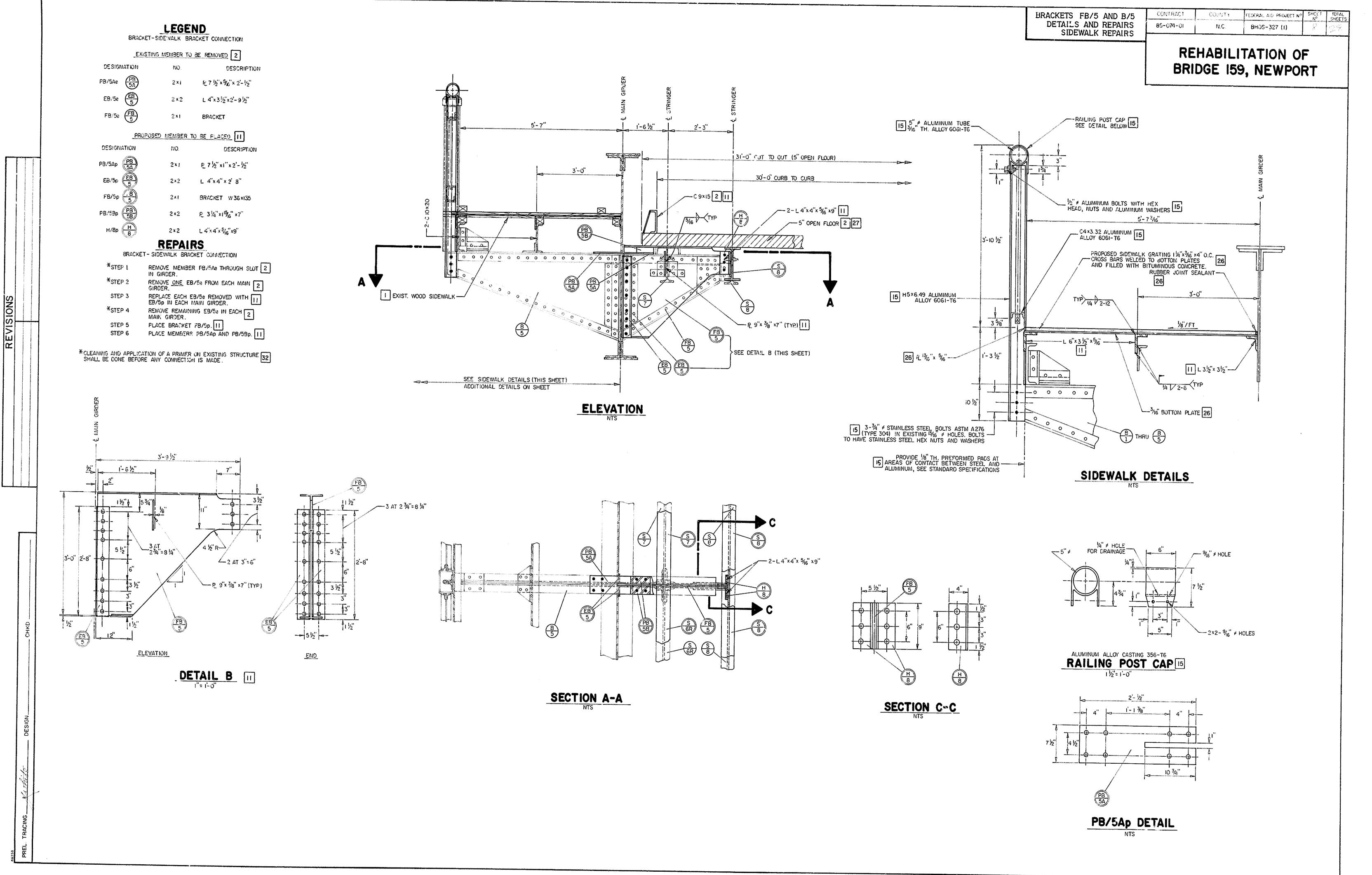
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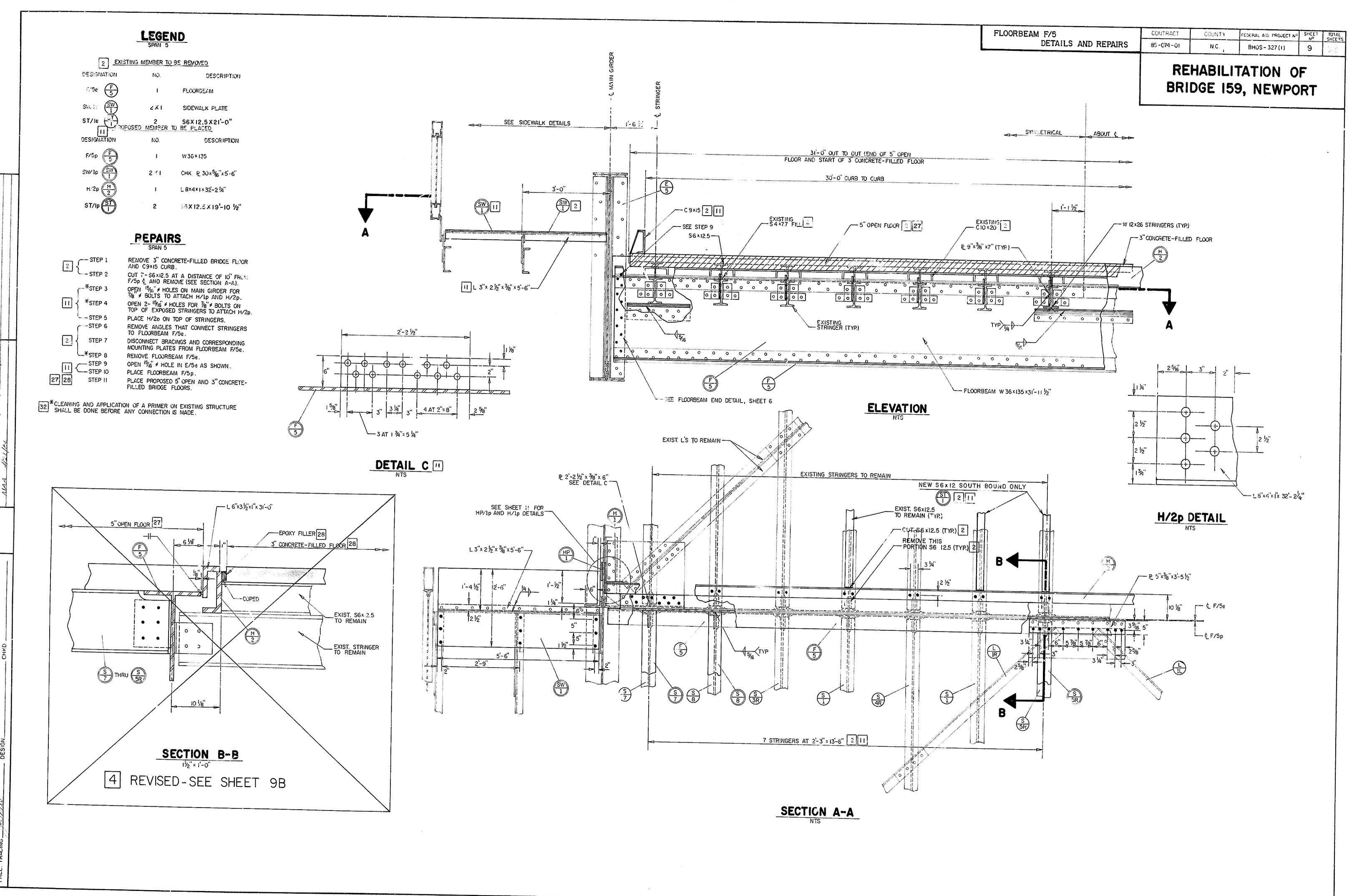
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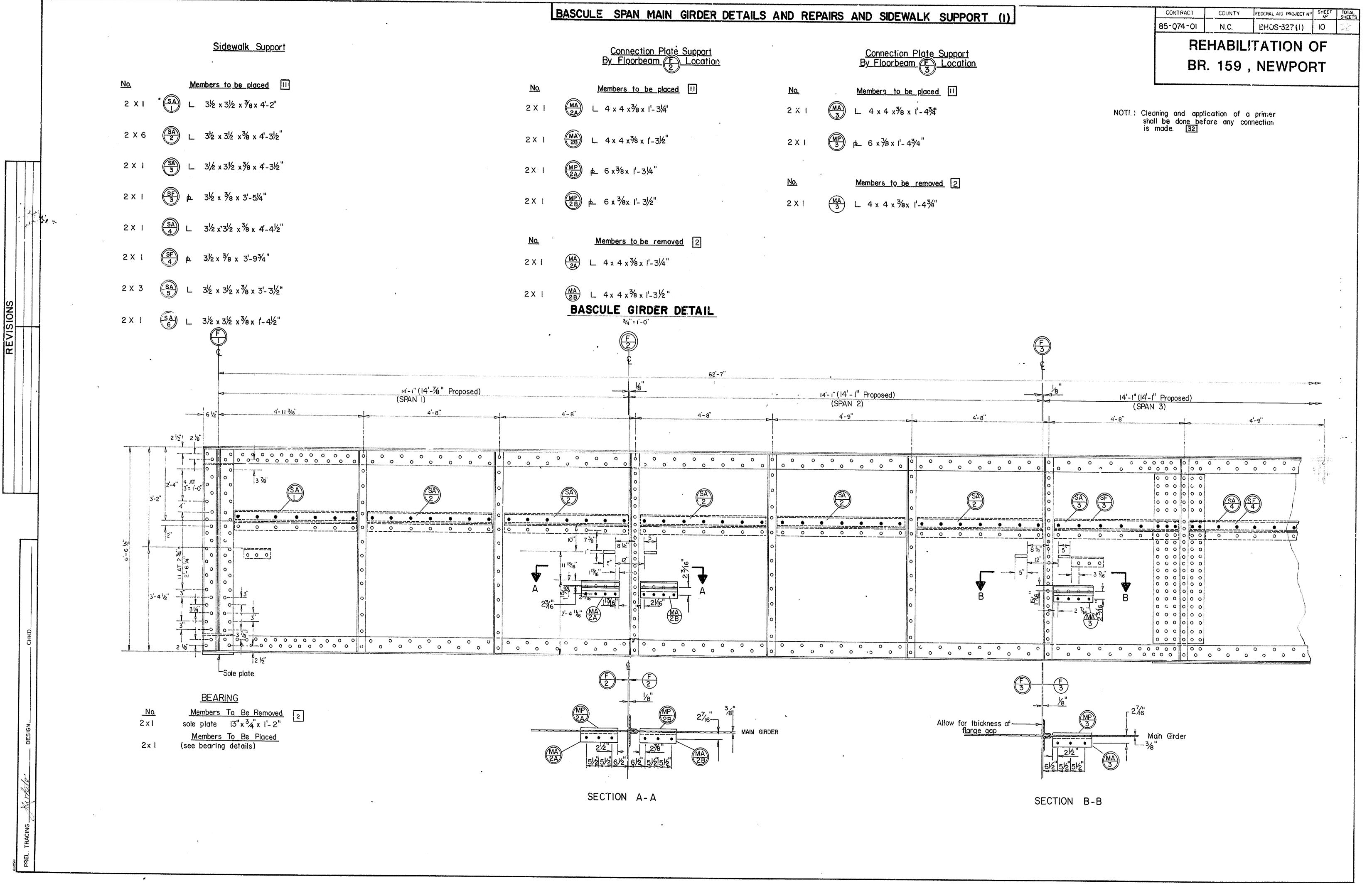


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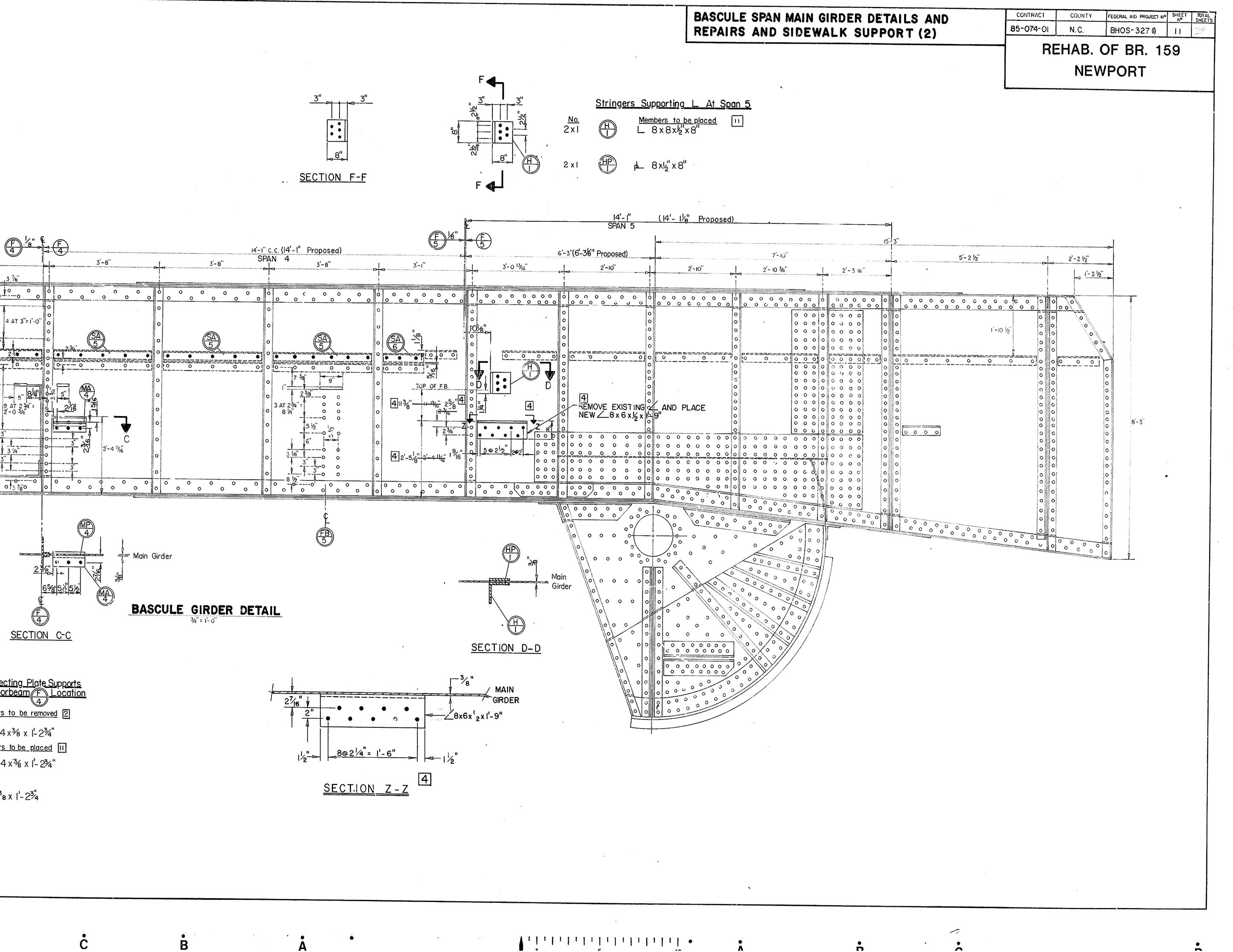
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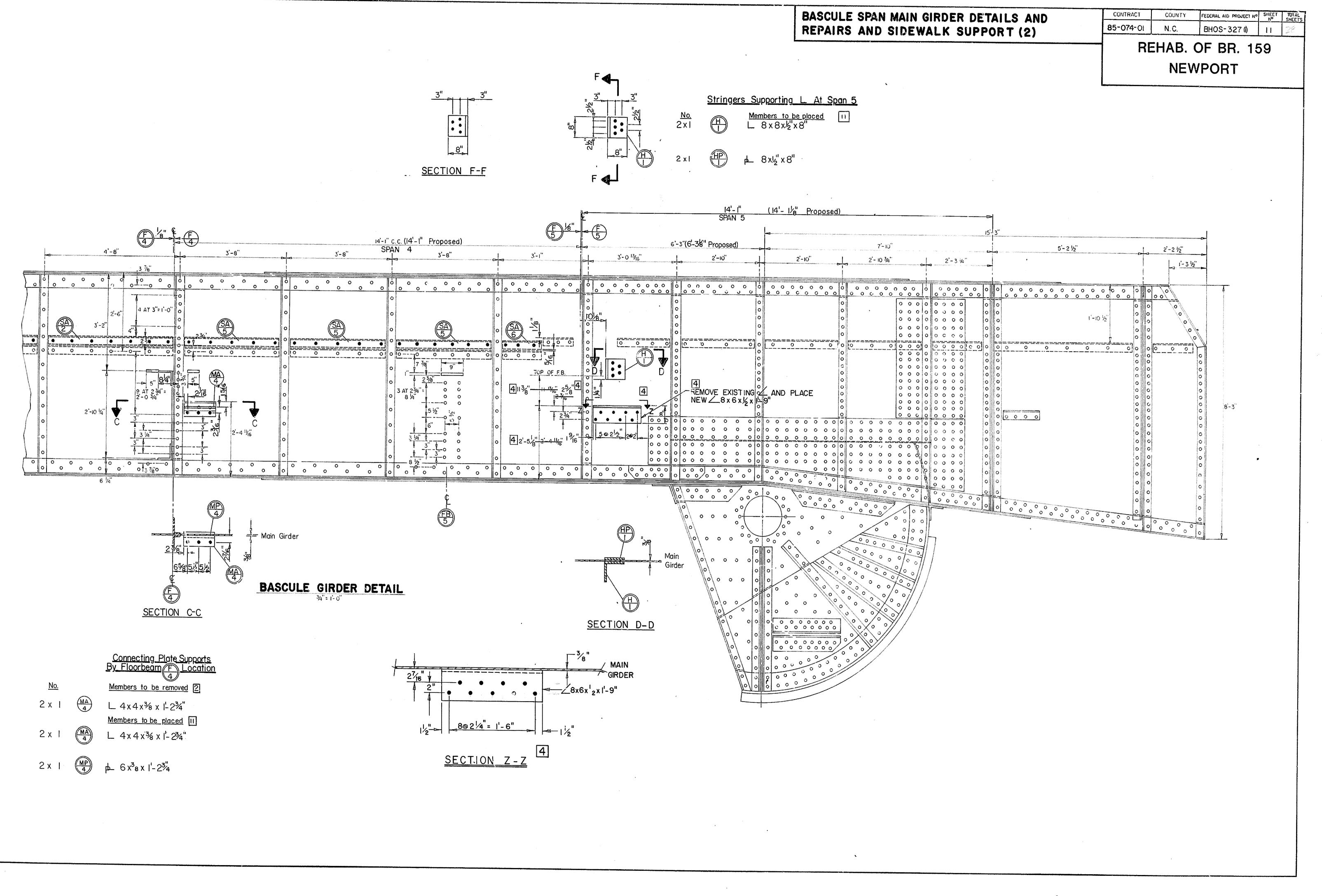


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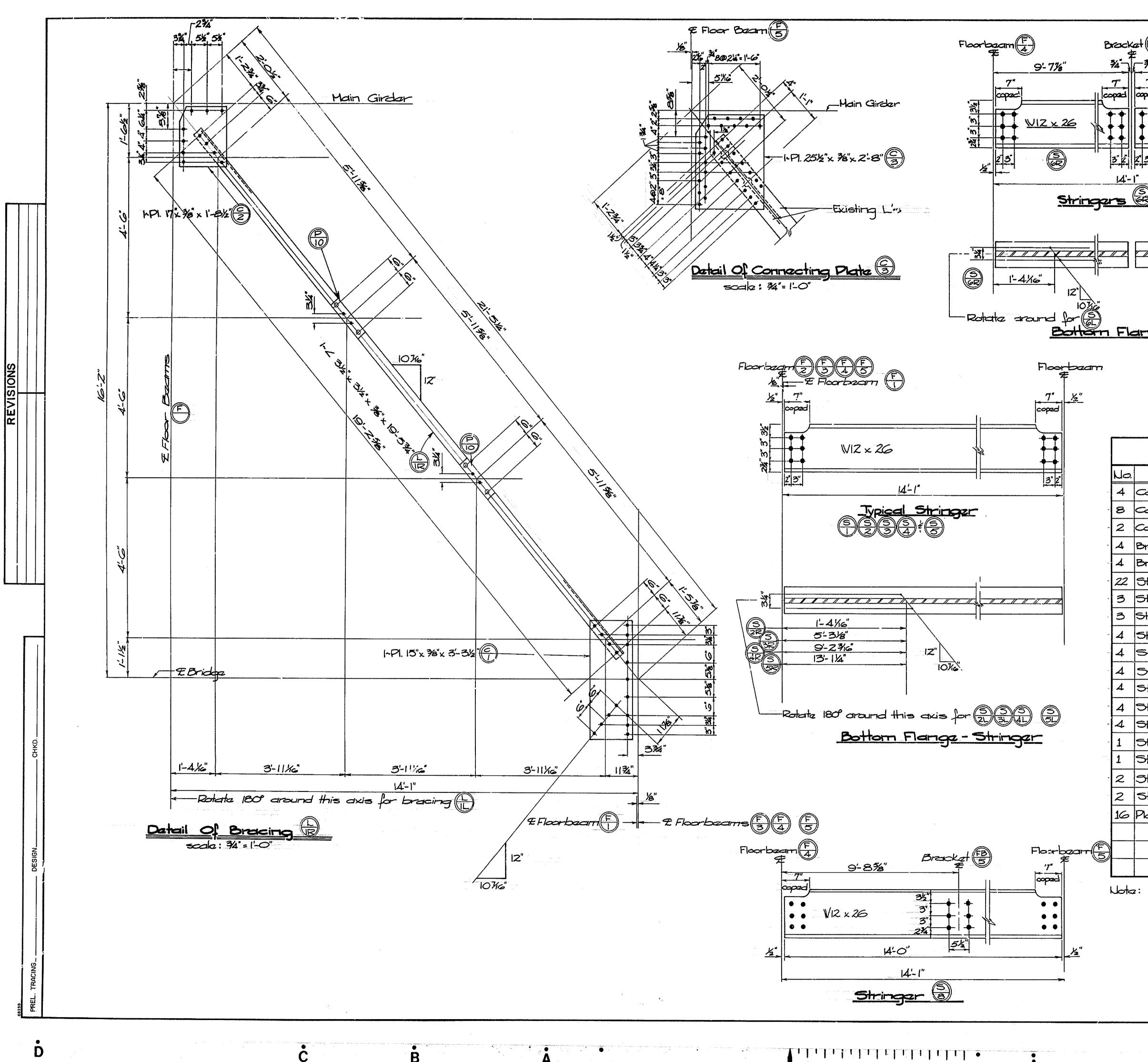
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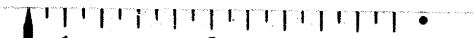
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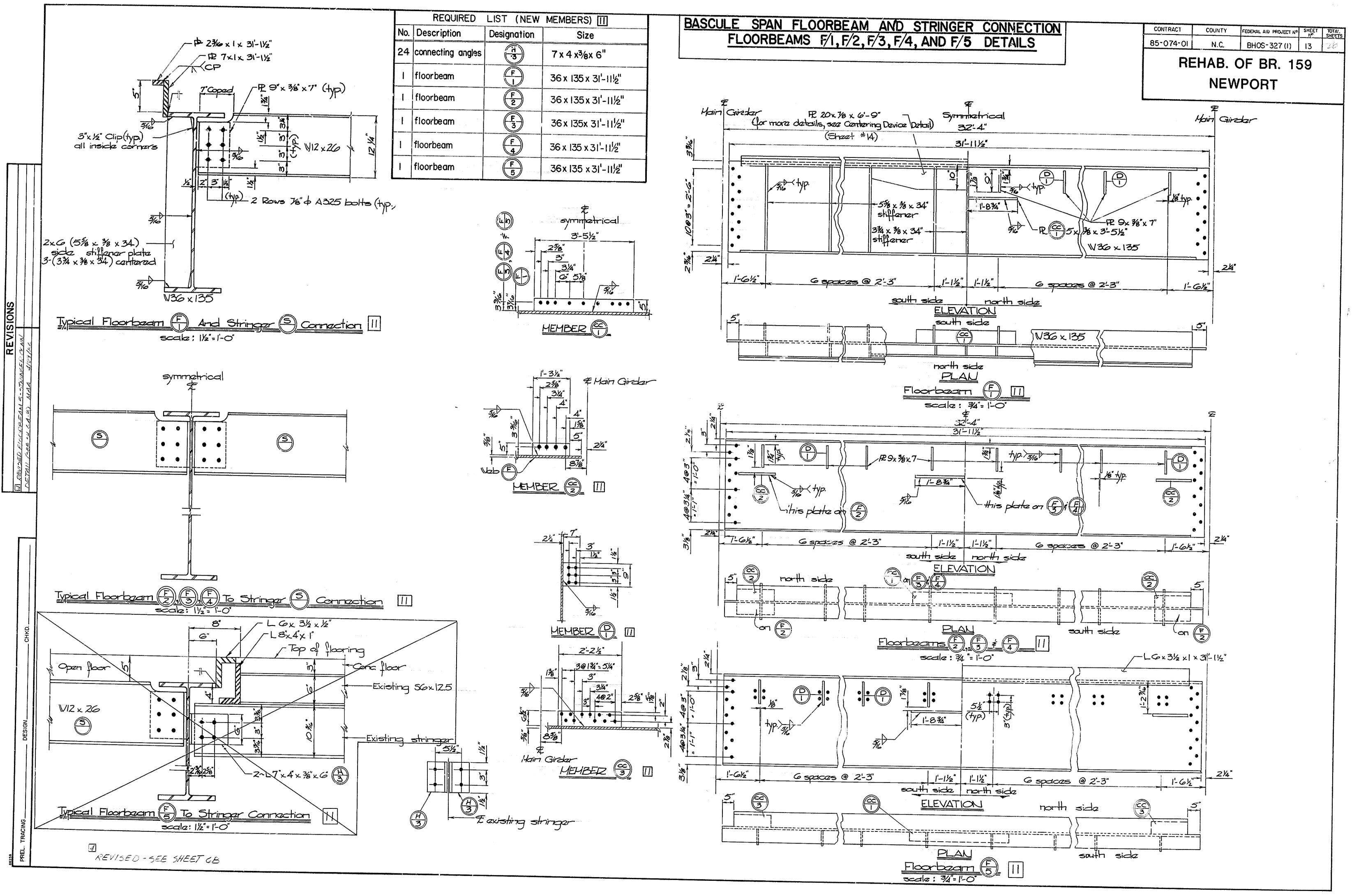


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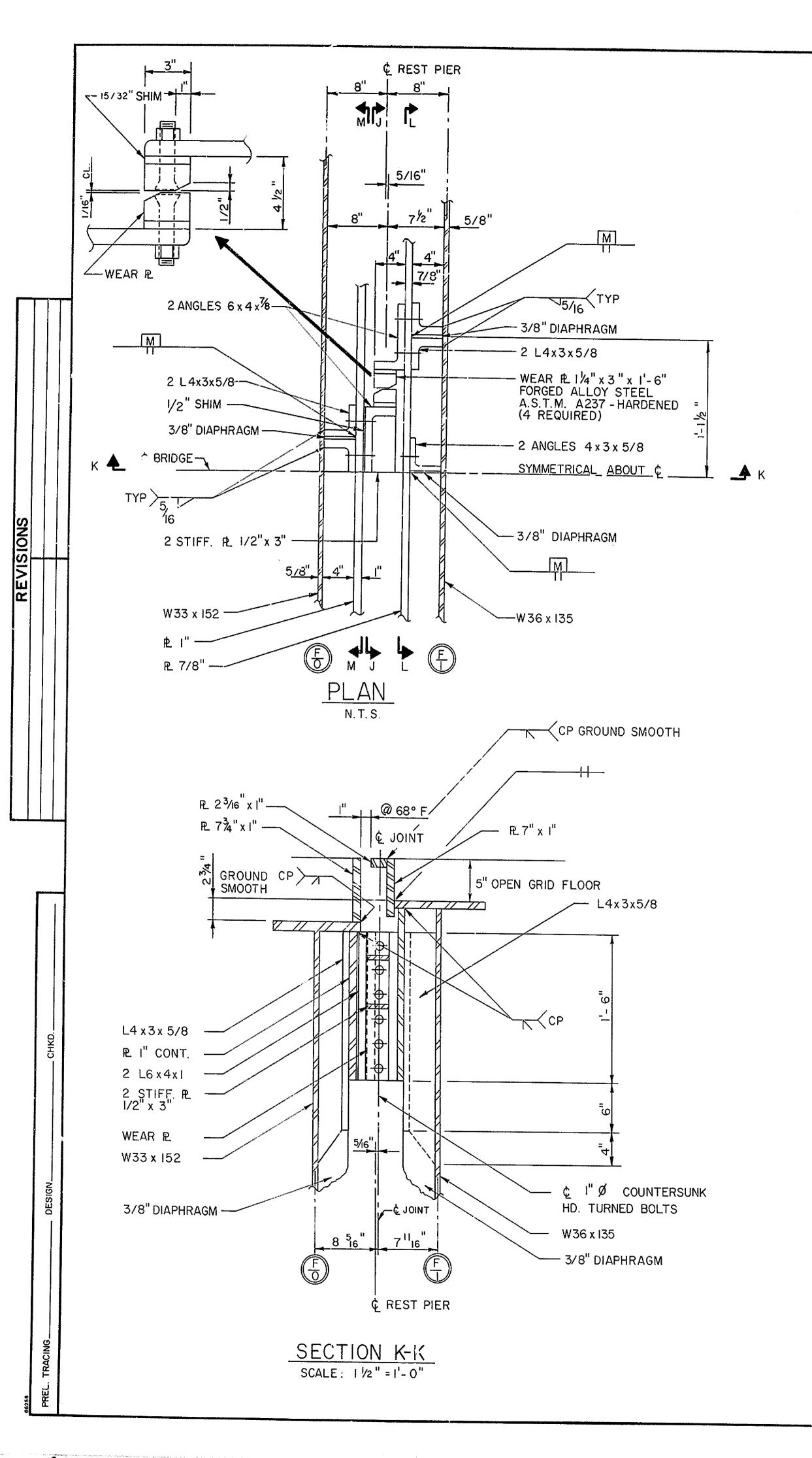
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Description princeting Plates princeting Plates princeting Plates pringers pringers ringers tringers tringers tringers			$\frac{5}{2-15} \times \frac{3}{5} \times \frac{3}{5} - \frac{3}{2}$ $2-17 \times \frac{3}{5} \times \frac{3}{5} - \frac{3}{2}$ $2-17 \times \frac{3}{5} \times \frac{1}{5} - \frac{3}{2}$ $2-25 \times \frac{3}{5} \times \frac{3}{5} \times \frac{1}{5} - \frac{3}{5}$ $3\frac{1}{2} \times \frac{3}{2} \times \frac{3}{5} \times \frac{19}{5} \times \frac{19}{5} - \frac{5}{5}$ $3\frac{1}{2} \times \frac{3}{2} \times \frac{3}{5} \times \frac{19}{5} \times \frac{19}{5} - \frac{5}{5}$ $\frac{3}{2} \times \frac{3}{2} \times \frac{3}{5} \times \frac{19}{5} \times \frac{19}{5} - \frac{5}{5}$ $\frac{112 \times 26 \times 14^{-0}}{112 \times 26 \times 14^{-0}}$ $\frac{112 \times 26 \times 14^{-0}}{112 \times 26 \times 14^{-0}}$ $\frac{112 \times 26 \times 14^{-0}}{112 \times 26 \times 14^{-0}}$ $\frac{112 \times 26 \times 14^{-0}}{112 \times 26 \times 14^{-0}}$	5¾" 5¾'
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Description princeting Plates princeting Plates acting	ANE NEW ROLL AND RO		$\frac{5}{2} = 15^{\circ} \times 36^{\circ} \times 3^{\circ} - 3\frac{1}{2}^{\circ}$ $2 - 17^{\circ} \times 36^{\circ} \times 1^{\circ} - 8\frac{1}{2}^{\circ}$ $2 - 25\frac{1}{2}^{\circ} \times 36^{\circ} \times 1^{\circ} - 8\frac{1}{2}^{\circ}$ $3\frac{1}{2}^{\circ} \times 3\frac{1}{2}^{\circ} \times 36^{\circ} \times 19^{\circ} - 8$ $3\frac{1}{2}^{\circ} \times 3\frac{1}{2}^{\circ} \times 36^{\circ} \times 19^{\circ} - 8$ $12 \times 26 \times 14^{\circ} - 0^{\circ}$ $12 \times 26 \times 9^{\circ} - 7\frac{3}{8}^{\circ}$ $12 \times 26 \times 4^{\circ} - 3\frac{16^{\circ}}{8}$	5 34"
Description princeting Plates princeting Plates acings aci			$\frac{5}{2} = 15^{\circ} \times 36^{\circ} \times 3^{\circ} - 34^{\circ}$ $2 - 17^{\circ} \times 36^{\circ} \times 1^{\circ} - 84^{\circ}$ $2 - 25 + 2^{\circ} \times 38^{\circ} \times 2^{\circ} - 8^{\circ}$ $3 + 2^{\circ} \times 34^{\circ} \times 34^{\circ} \times 19^{\circ} - 8^{\circ}$ $3 + 2^{\circ} \times 34^{\circ} \times 34^{\circ} \times 19^{\circ} - 8^{\circ}$ $4 + 2^{\circ} \times 34^{\circ} \times 34^{\circ} \times 14^{\circ} - 0^{\circ}$ $4 + 2^{\circ} \times 4^{\circ} - 3^{\circ} \times 14^{\circ} - 0^{\circ}$ $4 + 2^{\circ} \times 4^{\circ} - 3^{\circ} \times 14^{\circ} - 0^{\circ}$ $4 + 2^{\circ} \times 4^{\circ} - 3^{\circ} \times 14^{\circ} - 0^{\circ}$ $4 + 2^{\circ} \times 4^{\circ} - 3^{\circ} \times 14^{\circ} - 0^{\circ}$ $4 + 2^{\circ} \times 4^{\circ} - 3^{\circ} \times 14^{\circ} - 0^{\circ}$	5 34"
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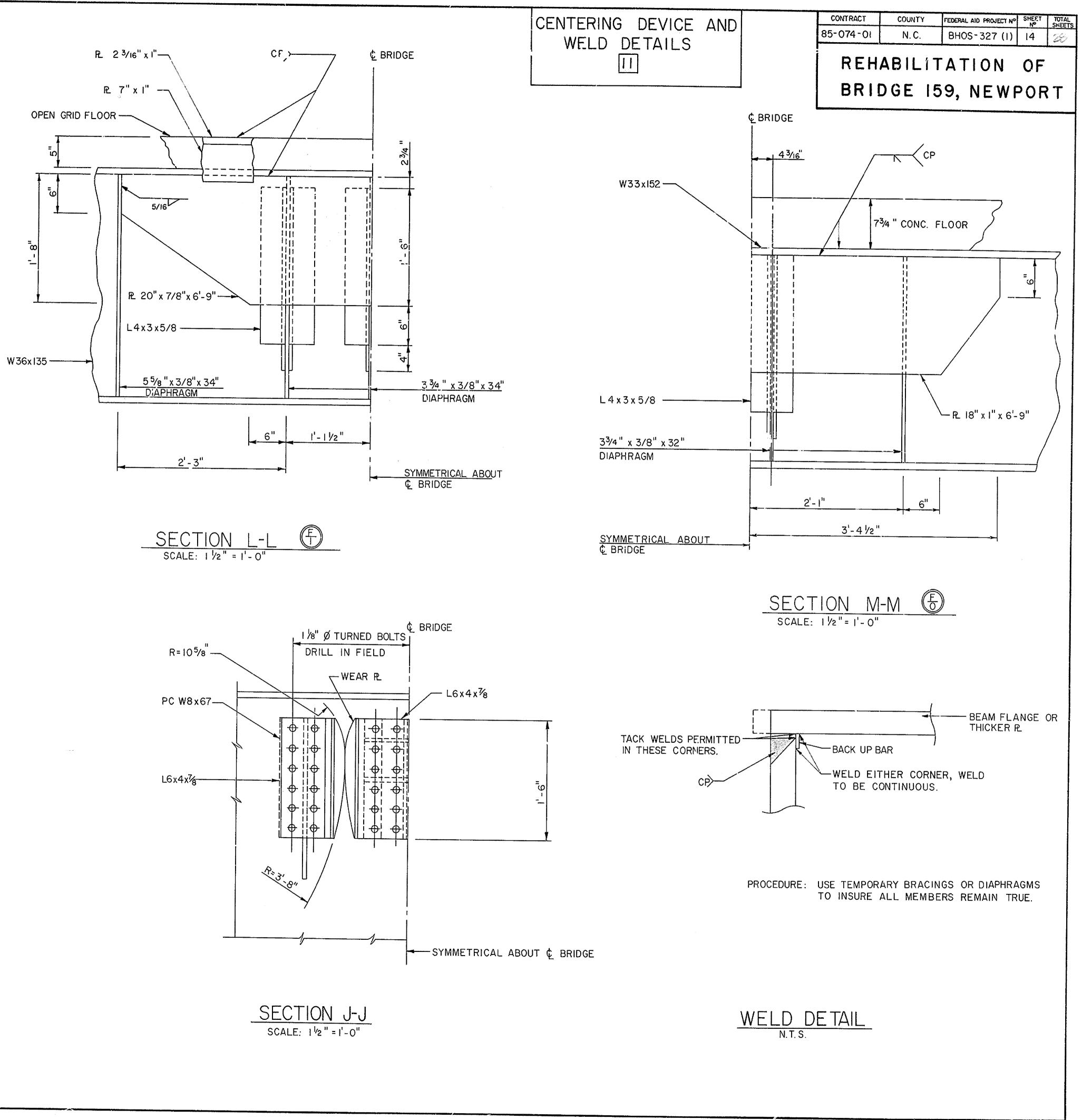
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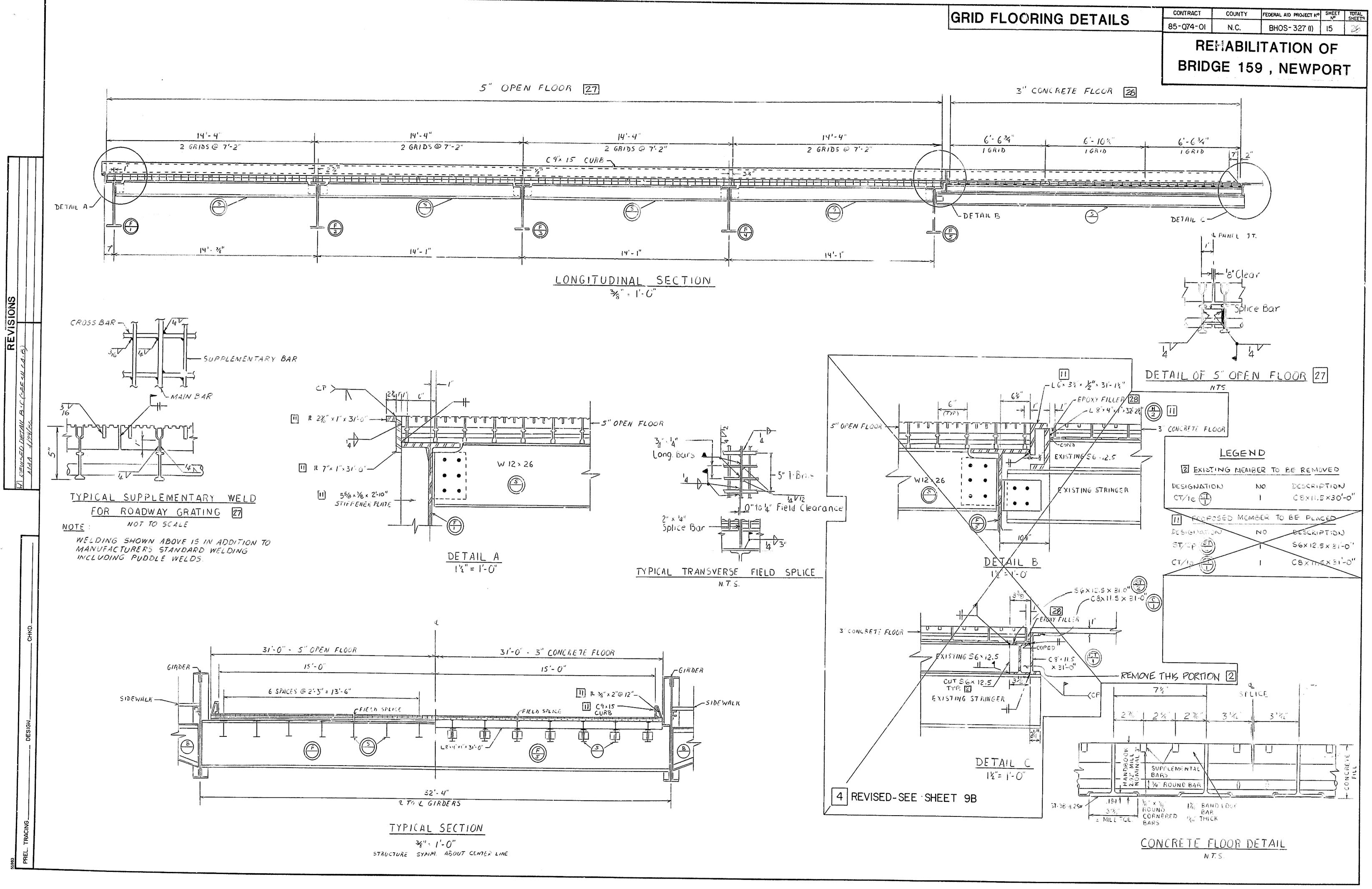


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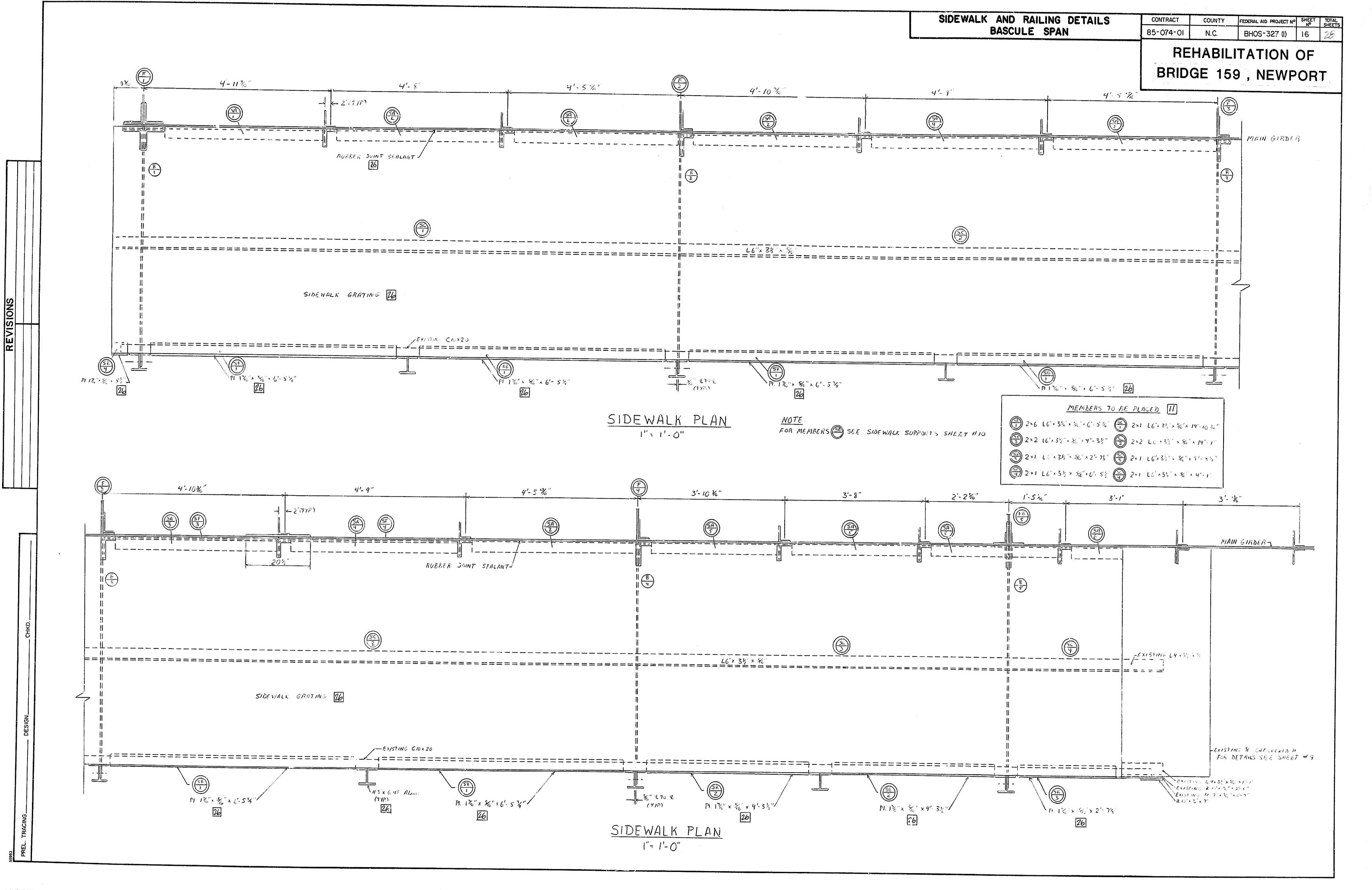
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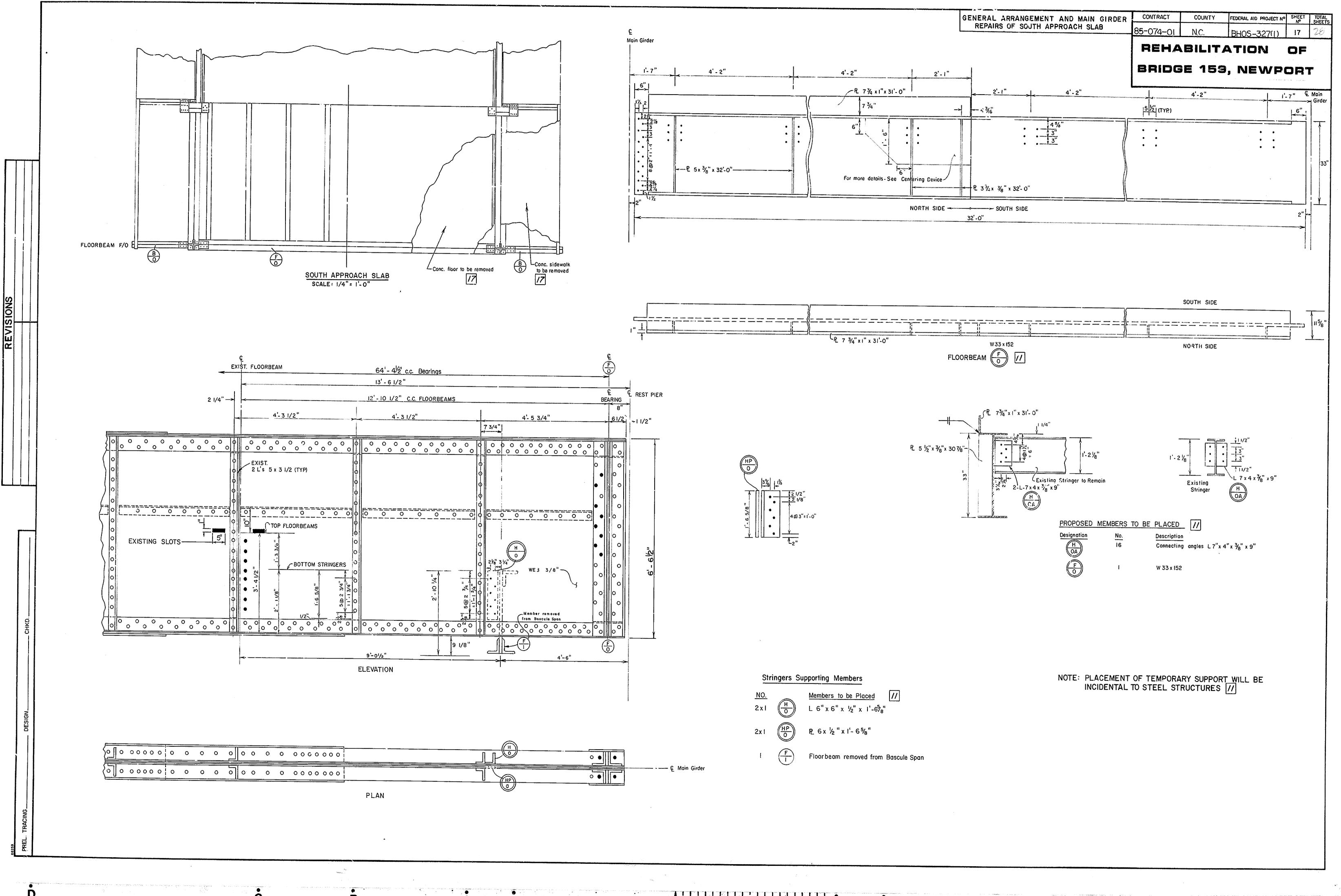
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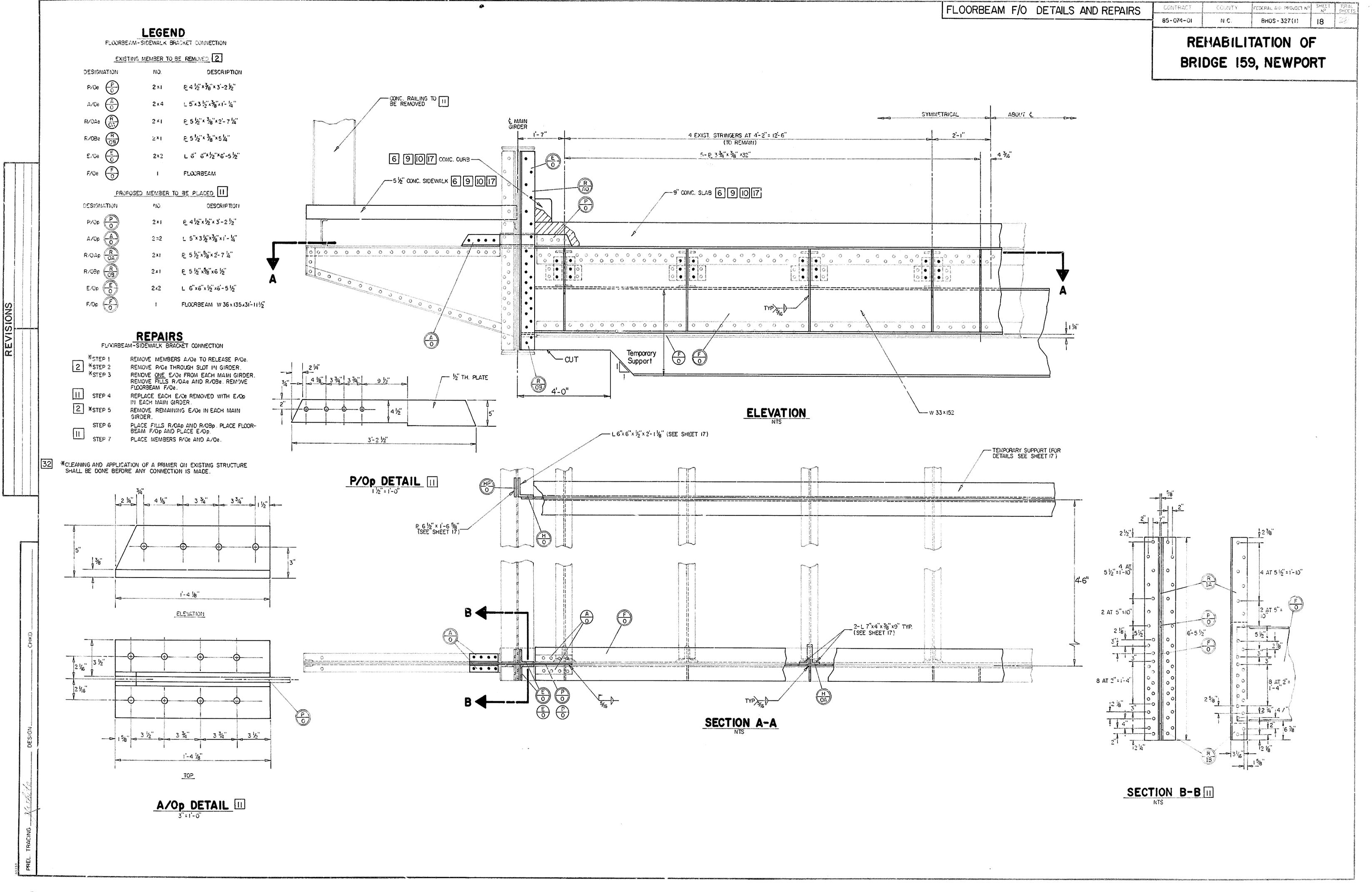


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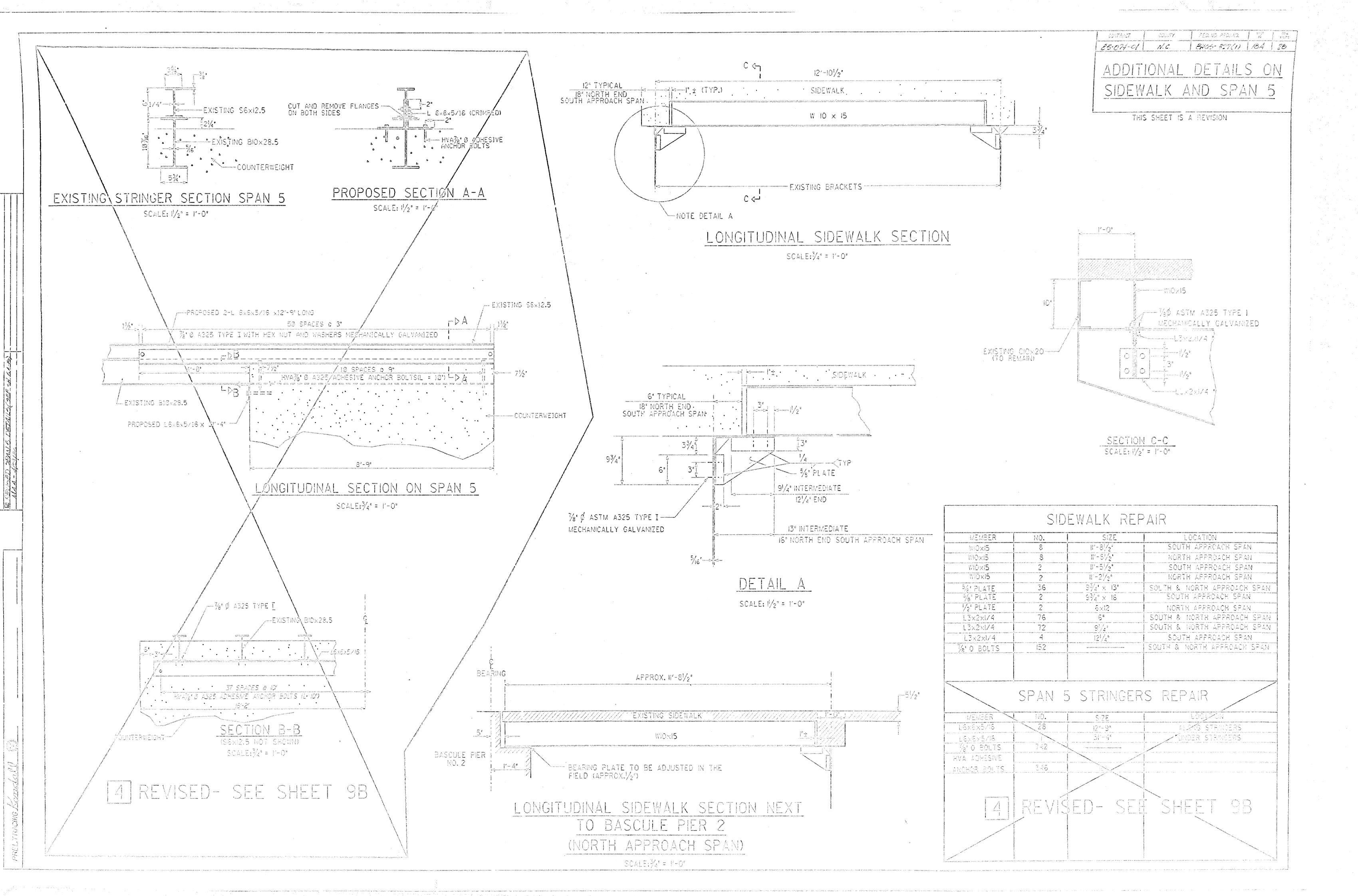
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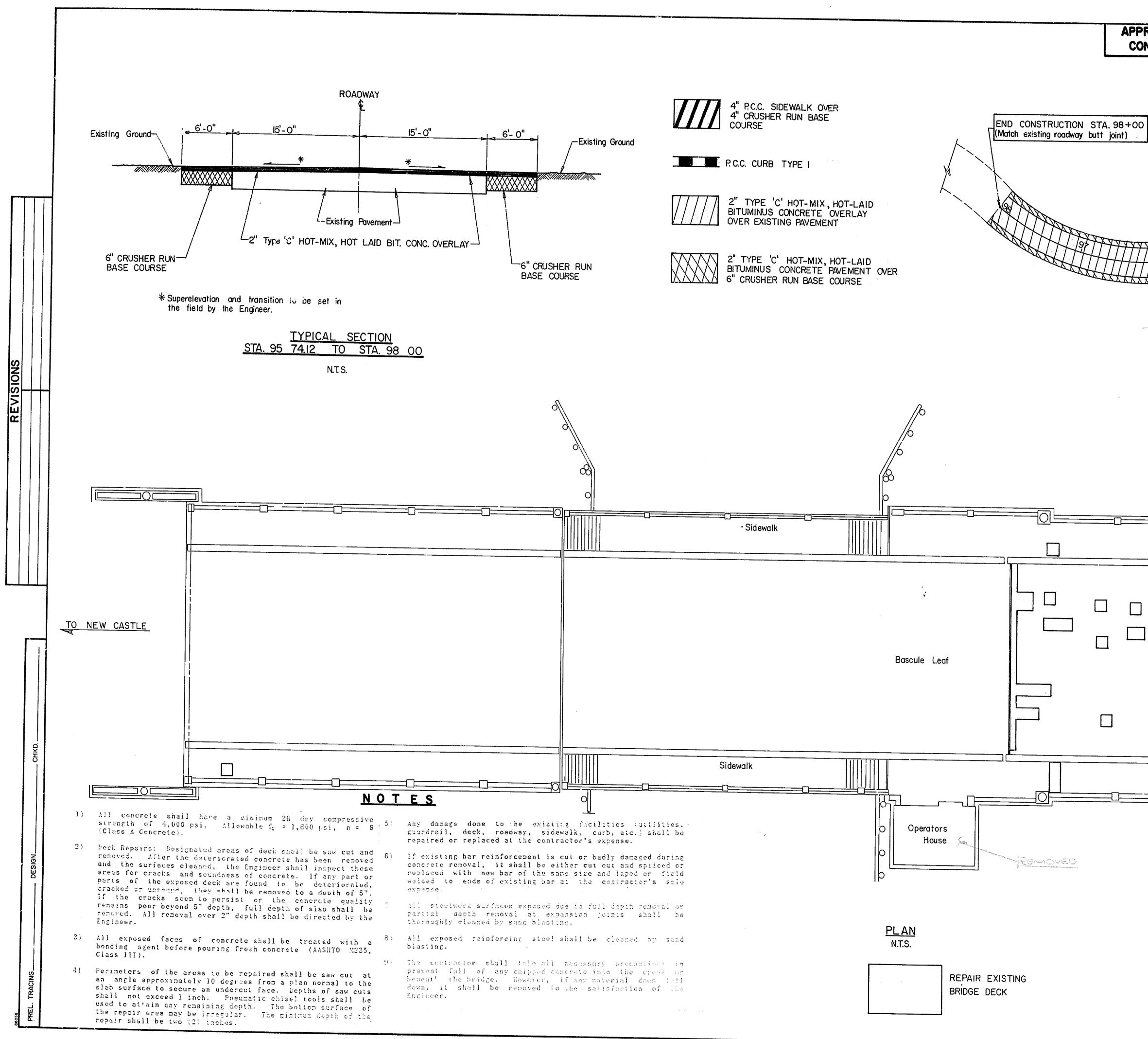
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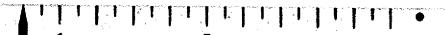
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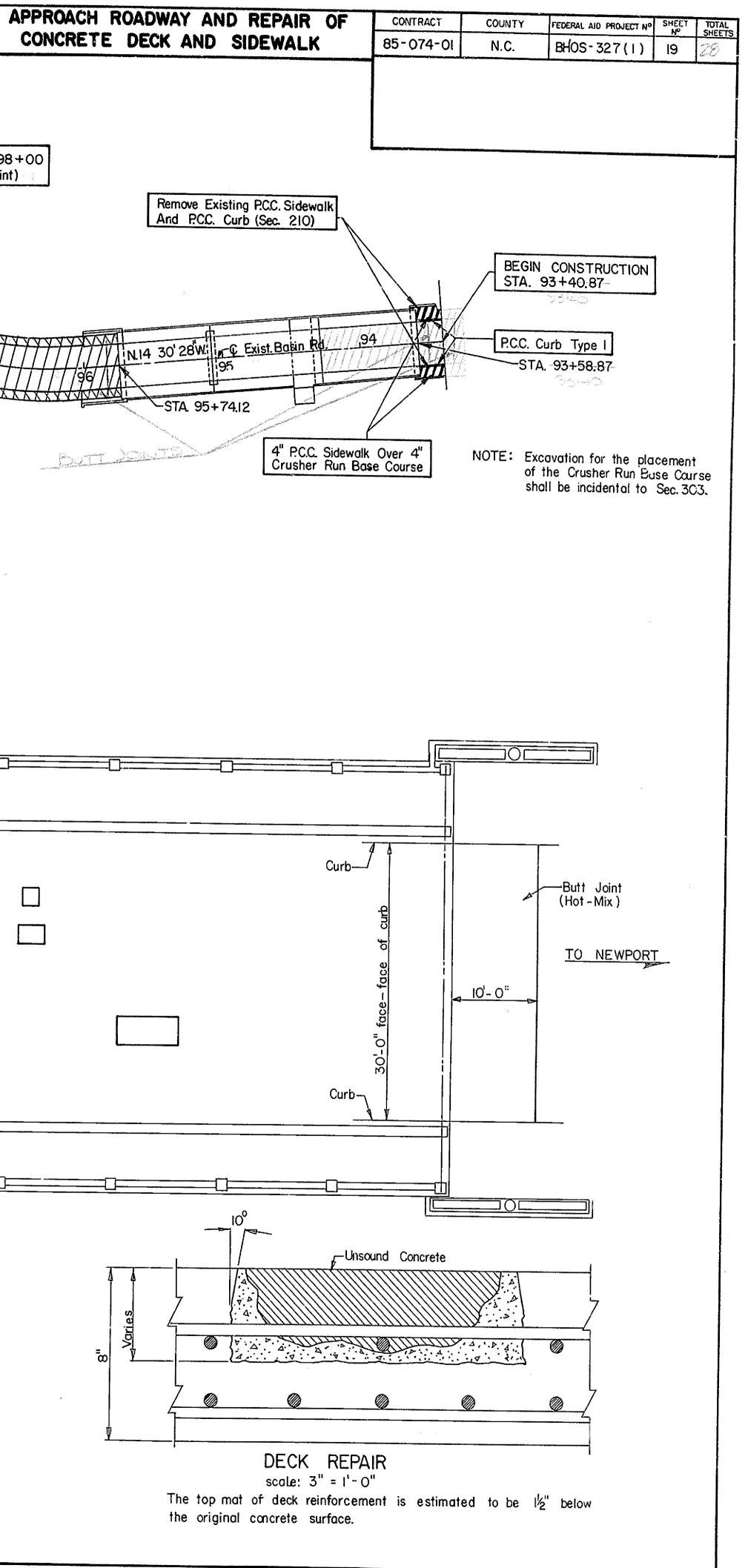




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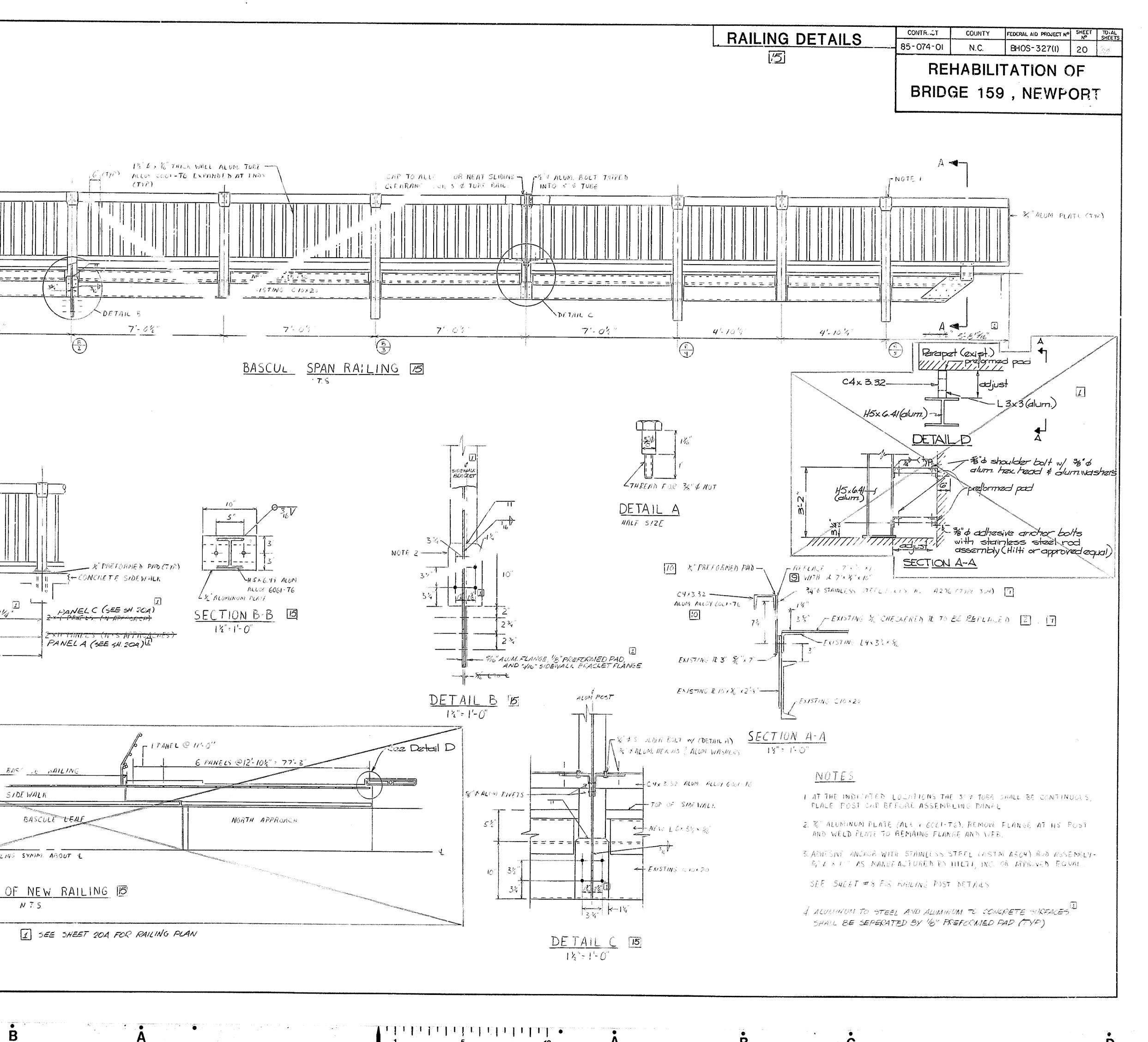


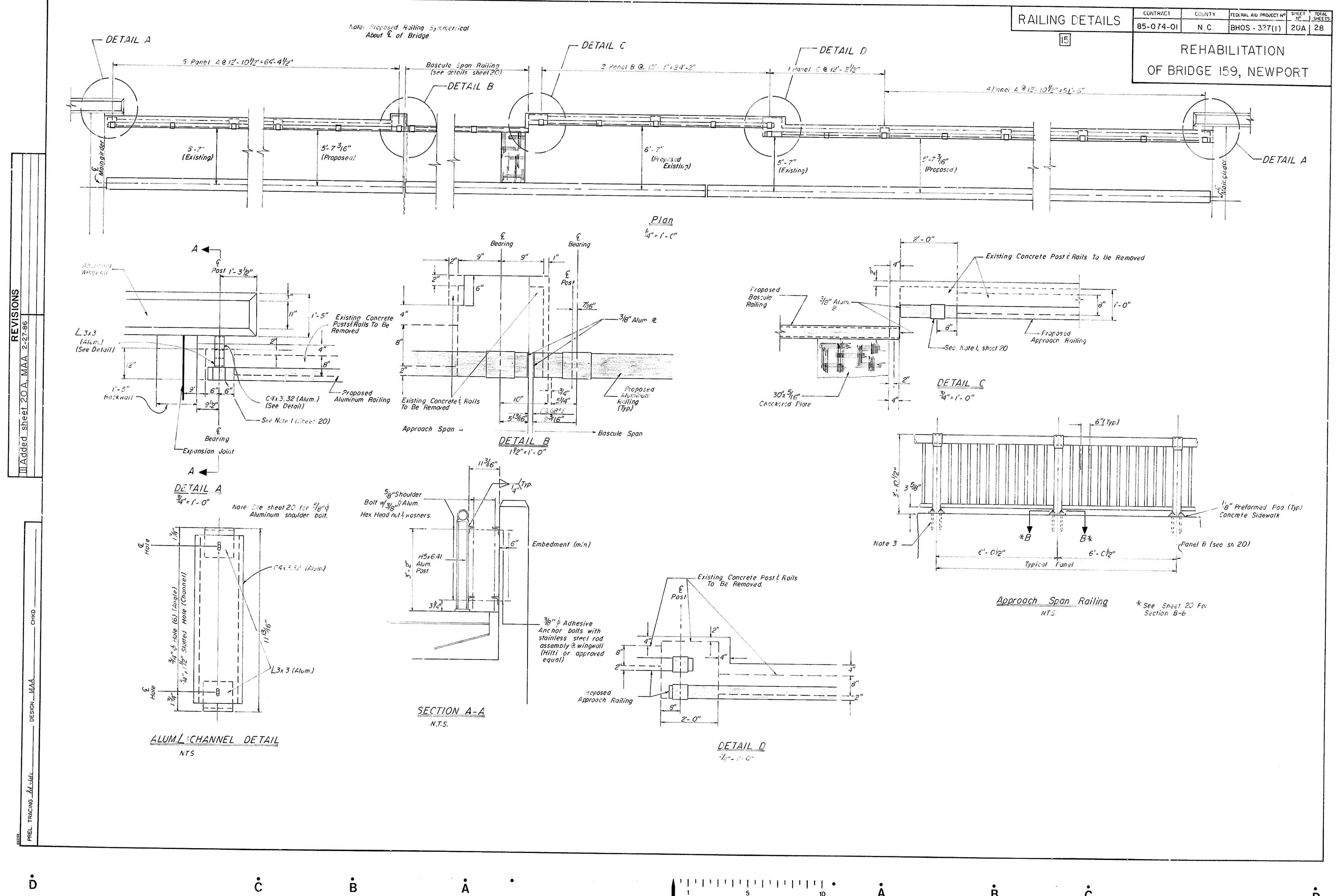
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FNOTE I 6 CTYPY CARGEN ANALYSIS STARTS COMPANY AND AND ANALYSIS ANALYSIS ANALYSIS ANALYSIS anana an Anan DETAIL B EB I 7' 0% 7'-0'2" $\binom{\mathbb{E}}{2}$ REVISIONS _____ (TES {← CONCRETE SIDEWALK **____**PL3'×≶ं× (∂`_ NOTE 3-HANELC (SEE SH 20A) 5-6 6-14" 5 6-14 -P-HNE-E-5----A-APHAR 6-54" 6-54 PANELA (SEE 54. 204) TIFICAL PANEL APPROACH SPAN RAILING 15 NTS See Detail D 5 RANGES @12-104" = 64'-44" BASS JE NAILING SIDE WALK SOUTH APPROAKH BASCULTEFALE RAILING SYMM. ABOUT 4 PLAN OF NEW RAILING 15 N 7.5 1 SEE SHEET 204 FOR RAILING PLAN

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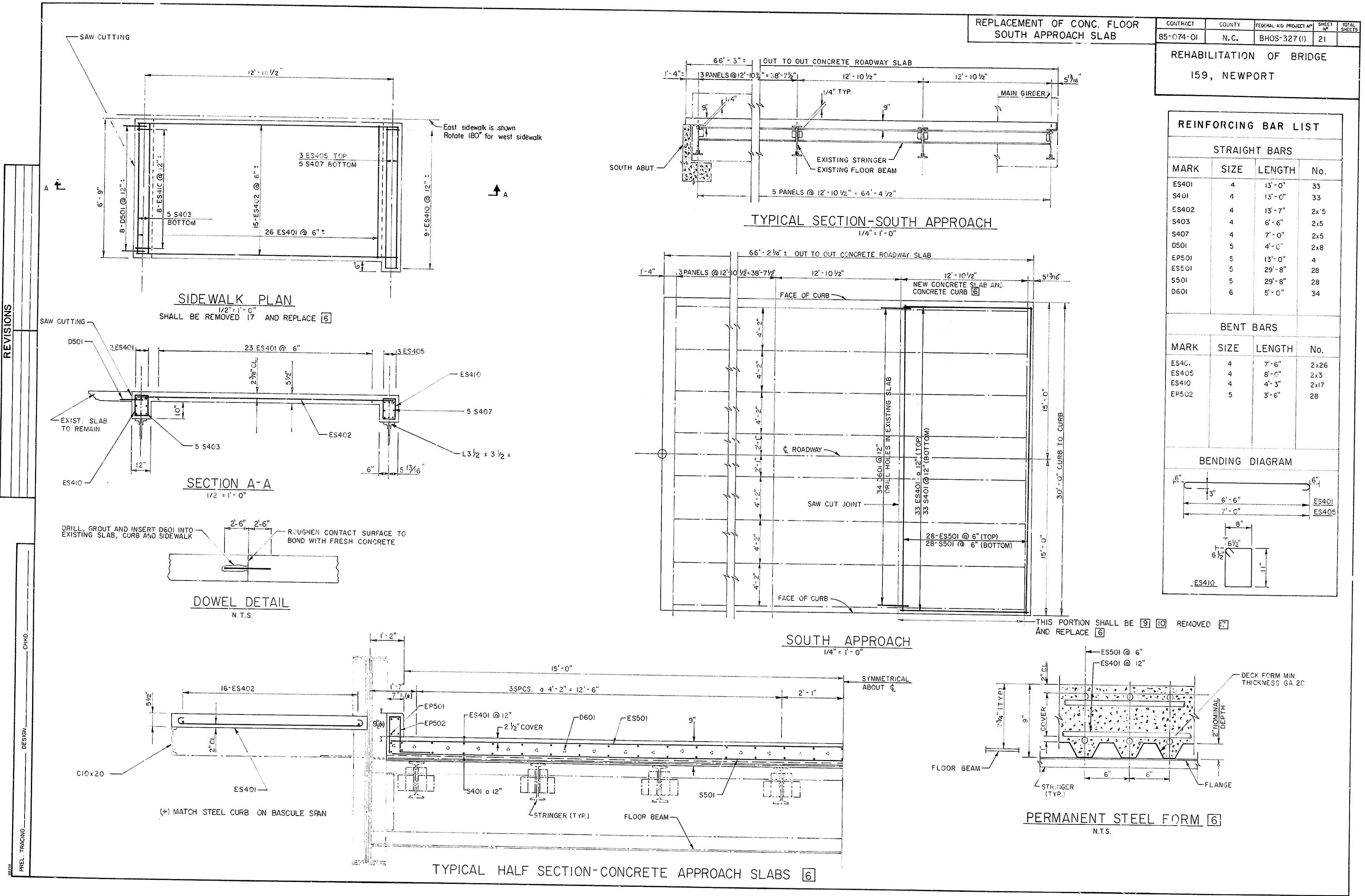
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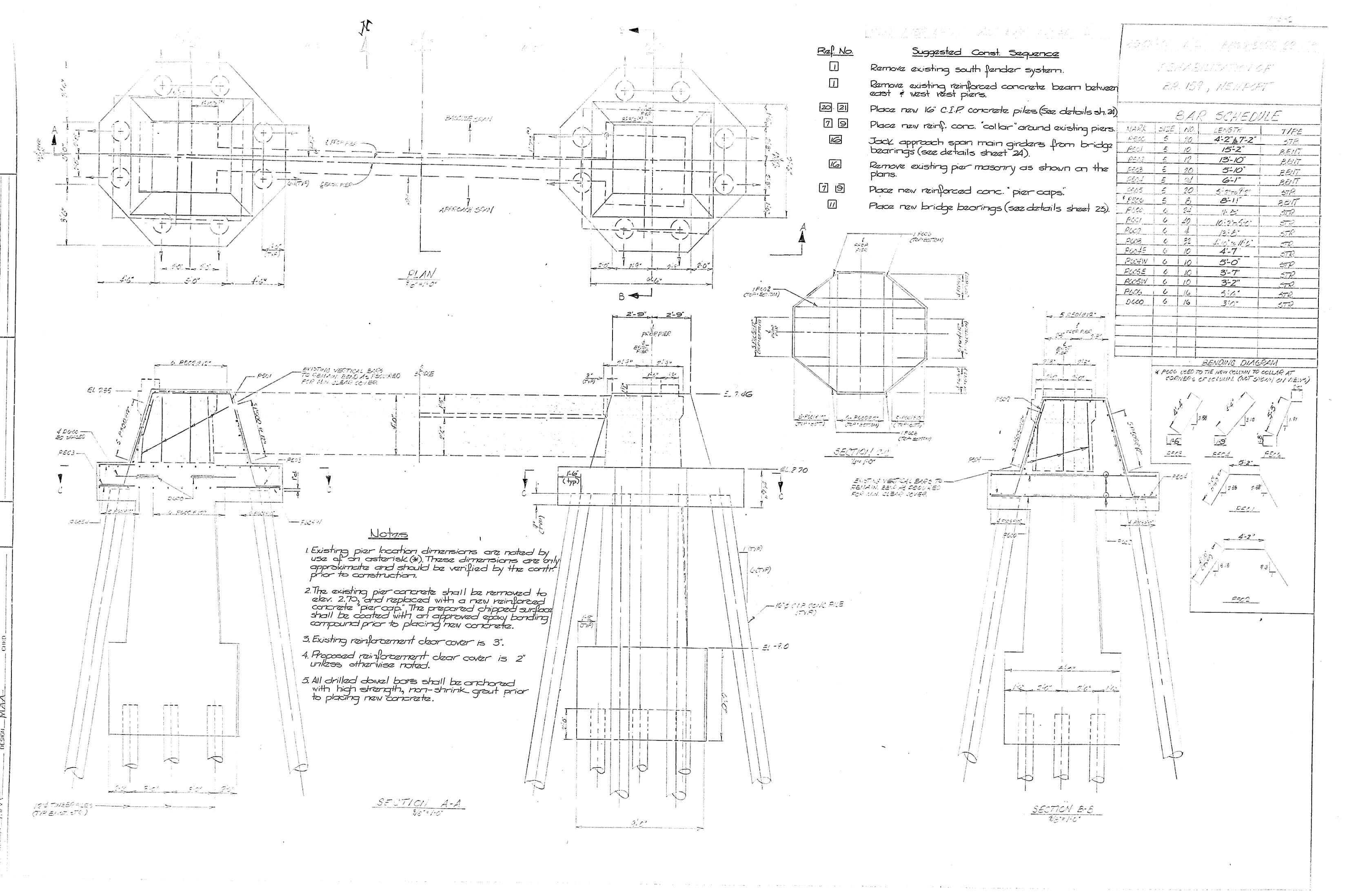


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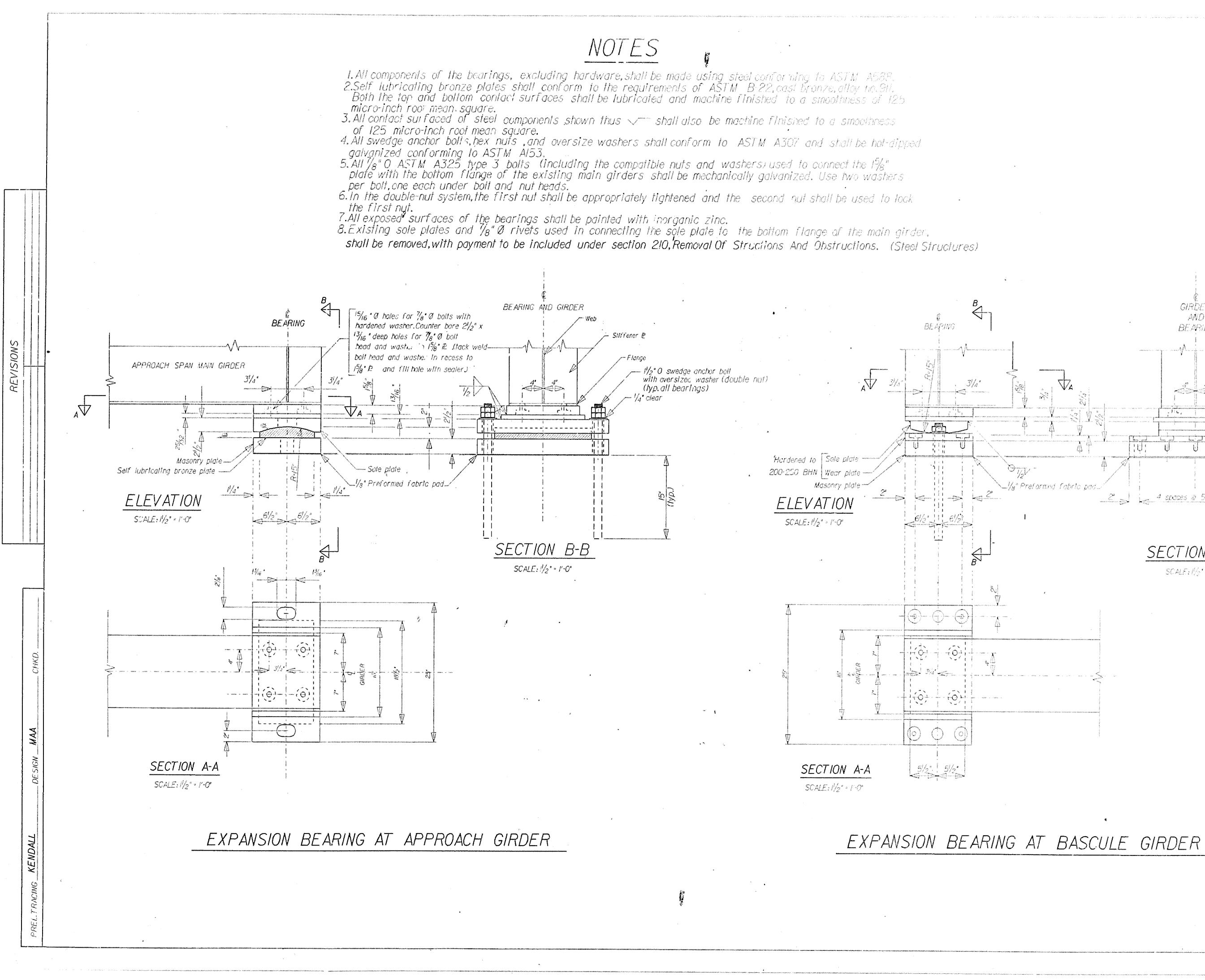
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85-061 ar N.C. RHOS-32700 23 28 REHAB. OF BR. 159 NEWPORT GIRDER AND BEARING , Counter bore holes 2½°0 x¾° deep in 1/₄° & for A325 cap screws the, pile, ¾ 0 x 1¾ long tapped hole in masonry plate.Hardened washer.Fill hole with sealer. 1:0: - 21/8" @ hole for 11/2" @ swedge anctor balt(not shown) 2 3 is a spaces a 51/2° - 2° 3 is 22 SECTION B-B SCALE: 11/2* - 1-0"

			15-074-01 Bridge No. 159 (Newport Bascule Bridge)
BO	SAMP		uth of southeast corner of south abutment, 15'from & bridge d
No.	Depth	Blows/6"	SAMPLE DESCRIPTION
	1.0'	3 4	Moist firm brown clayey course to fine sar silt and gravel.
	3.0'	3 4	I4" Recovery
2	4.0'	3 2 7	Moist firm brown clayey silt w/some fine sand, trace of gravel.
	6.0'	3 4	8" Recovery
3	9.0'	76	Wet stiff brown fine to course sandy silt w/trace of gravel, trace of organic
	11.0	5 4	matter. 11" Recovery
4	14.0'	2 2 3 3	Saturated firm brown silty fine to course sandy clay w/organic matter.
	16.0'	3 3	12" Recovery
5	19.0'	2	Saturated soft gray clayey course to fine sandy silt, w/trace of gravel, trace
	21.0'	2	of organic matter.
6	24.0'		Saturated soft gray silty organic clay w/trace of fine sand.
	26.0'		23" Recovery
7	29.0		Saturated firm gray silty fine to course sandy clay w/trace of gravel, trace of
	31.0	2 6	organic matter.
8	34.0'	13 18	Moist dense brown and gray course to fine sand and gravel w/trace of silt.
	35.5'	15	18" Recovery
9	39.0'	11 9	Moist very stiff multicolored silty fine to course sandy clay.
	40.5'	20	18" Recovery
10	44.0'	7 5	Moist very stiff multicolored clayey fine to course sandy silt.
	46.0'	 0	23" Recovery
	49.0'	6 10	Moist hard multicolored clayey fine to course sandy silt w/trace of gravel.
	51.0'	21 31	20" Recovery

STANDARD

Wt. of samp

Average fall

Type of D-S

O.D. of San

Casing Size



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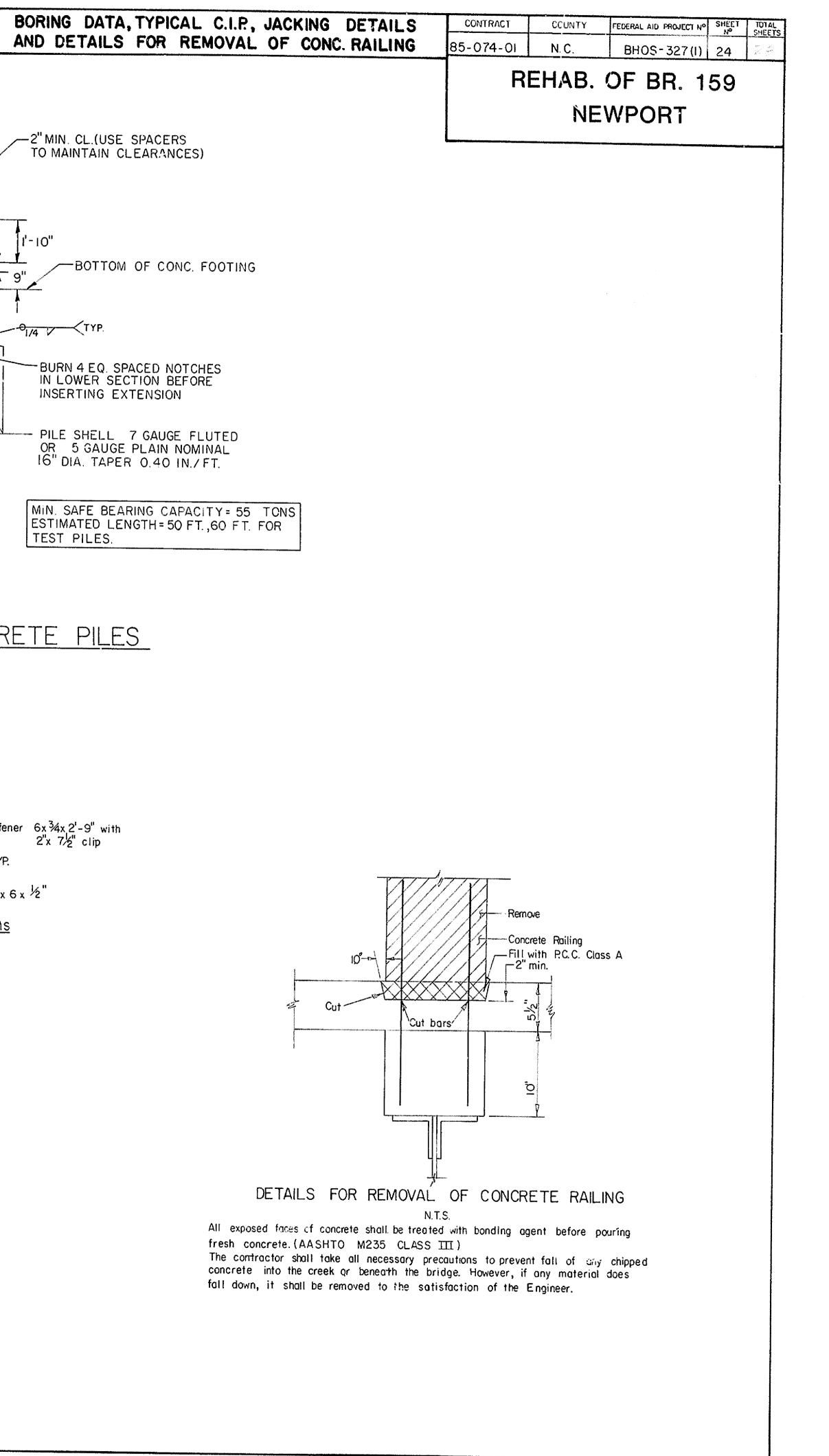
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REVISIONS



				AND	UETAILS FUR
PENETRATION					
nple hammer	140 lbs.	16" C.I.P. PILE	CONC.	-2" M TO M	IN. CL.(USE SPACE MAINTAIN CLEARAN
all	30.0 in.	6-#6 x			
-Sampler	Split Spoon	-EQUALL #3 TIES @ ۱ ا3-REQ'D. / ۱	Y SPACED	3" 1'-10"	BOTTOM OF CO
ampler	2 in.	13 - REQ D. / 1		9"	
e	3¼ in.		6" 434 MIN. 51/2"		TYP.
					IRN 4 EQ. SPACED N LOWER SECTION B SERTING EXTENSIO
				OR 16'	LE SHELL 7 GAUG 5 GAUGE PLAIN ' DIA. TAPER 0.40
				MiN EST	. SAFE BEARING CA IMATED LENGTH=5 T PILES.
		TYPICAL Trypical t from existing $\frac{F}{2}$ or $\frac{F}{3}$ of piers (Elev. 2.70)	N.T.S.		PILES
	4'0" <u>G DETAIL</u>	 Ø Piles			

<u>JACKING DETAIL</u> Scale: ³/4" = 1'-0"



10 B

	MEMBER	NO.	WEIGHT LBS/EA	H	M _H	SPAN]
	~"			75.54	687,414	1	
	5" OPEN FLOOR	1	9,100	61.46		AND	4
	(ONE SPAN)			47.37			* • •
		1		75.54			
	WOOD SIDEWALK	2	745	61.46		2	
	(ONE SPAN)			47.37		3	
				33.29			4
	S			75.54 61.46		2	
		14	400	47.37		3	1
				33.29		4	
				75.54	384,952]
		14	364	61.46	313,200		
				33.29	241,397	3 4	
				81.96	5,491	4	
	$\begin{pmatrix} C \\ I \end{pmatrix} OR \begin{pmatrix} C \\ I \end{pmatrix}$,	67	55.05	The second s	2	
		'	07	40.96		3	
				26.88	1,801	4	
				69.21	5,537	/	
	$\begin{pmatrix} c \\ 2 \end{pmatrix}$ OR $\begin{pmatrix} c \\ 2 \end{pmatrix}$	2	40	<u>67.79</u> 53.71	5,423	2 3	
				39.62	Contraction of the second s	4	
				75.54		/	
	$\left(\frac{L}{L}\right)$ OR $\left(\frac{L}{L}\right)$ + $\left(\frac{P}{L}\right)$	2	189	61.46	23,232	2	
				47.38		3	
				33.30 75.54		4	
	S4X7.7(FILL)	14	99	6!.46	85,184	2	
			55	47.37	65,655	3	
				33.29	46,140	4	
			07	82.58	A DESCRIPTION OF THE OWNER	/	
	CIO X 20 (FILL)	14	23	68.50 54.42		2 3	
				40.33			
			······································		······································		
	MEMBER	NO.	WEIGHT LBS./EA.	H	MH]	
	Ē		LBS./EA.			1	
	(7) + CA	1	4642	82.58	383,336		
	(\overline{F}) + C + CP + D	,	6477	00.50			
			6437	82.58	531,567		
	$\left(\frac{F}{2}\right)$ + CA	1	4694	68.50	321,539		
-		n a an	an a analana mandala kaominina mandra amin'ny fisiana ami	and BU . Budde billions on contract contract of physical and an and a second	an a man and an		
	2 + CP		4549	68.50	311,607		
		n doorantoonooniga ay ah an	nt allagitski (vijeka) (oblivna teoretan v dvalanski dolođena svore		San o'n an chwar gwe allen gwel a gan a gwel o gwel gwel a gw	-	
	3 + CA		4722	54.42	256,971		
	(7 <u>3</u> + CP	1	4574	54.42	248,917		
		- Mitte Madeinak zemen n. et en et en et fer fangelik genaam aan de fer	na febborasionamas na reprocesso non a processo na processo na processo na processo na processo na processo na	WMA-BAD INFO IN CONTRACTOR INFO A STATE OF A FINAL AT A STATE			
	(<u>↓</u> + CA		4722	40.33	190,438		
1	4 + CP				97 8 dia mila dia pika dia kaoka mangkao amin'ny fisiona amin'ny fisiona amin'ny fisiona amin'ny fisiona amin'n		
			4574	40.33	184,469		
	$\left(\frac{F}{5}\right)$ + CA	1	4752	26. 25	124,740		
		alle la de la companya de la company	MAN EVEN WHEN A MANAGEMENT AND				
	5 + CA+ CP	1	5140	26.25	134,925		
	$\frac{\overline{FB}}{5}$ + CA			21. da fan de sen an			
		2	386	20.58	15,888		
	(FB) 5 + CA+CP	2	337	30.62	20,638	C ₂ =	CENTERING
				50.02	20,000		
	SW or SW	2	226	26.41	11,937	· CA _z	FLOORBEAM - FLOORBEAM -
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		DECEMBER 1	1248	25.49	31,812	D =	DIAPHRAMS
	3"CONC.FLOOR	1	20500		200 000	CP=	FLOORBEAM -
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		an an ann an	950	54.10	102,790		
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REVISIONS

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UPPORTING ANGLES (SA) + (SB) + (SC)

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LOORBEAM - STRINGERS CONNECTING PLATES LOORBEAM - BRACINGS- CONNECTING PLATES

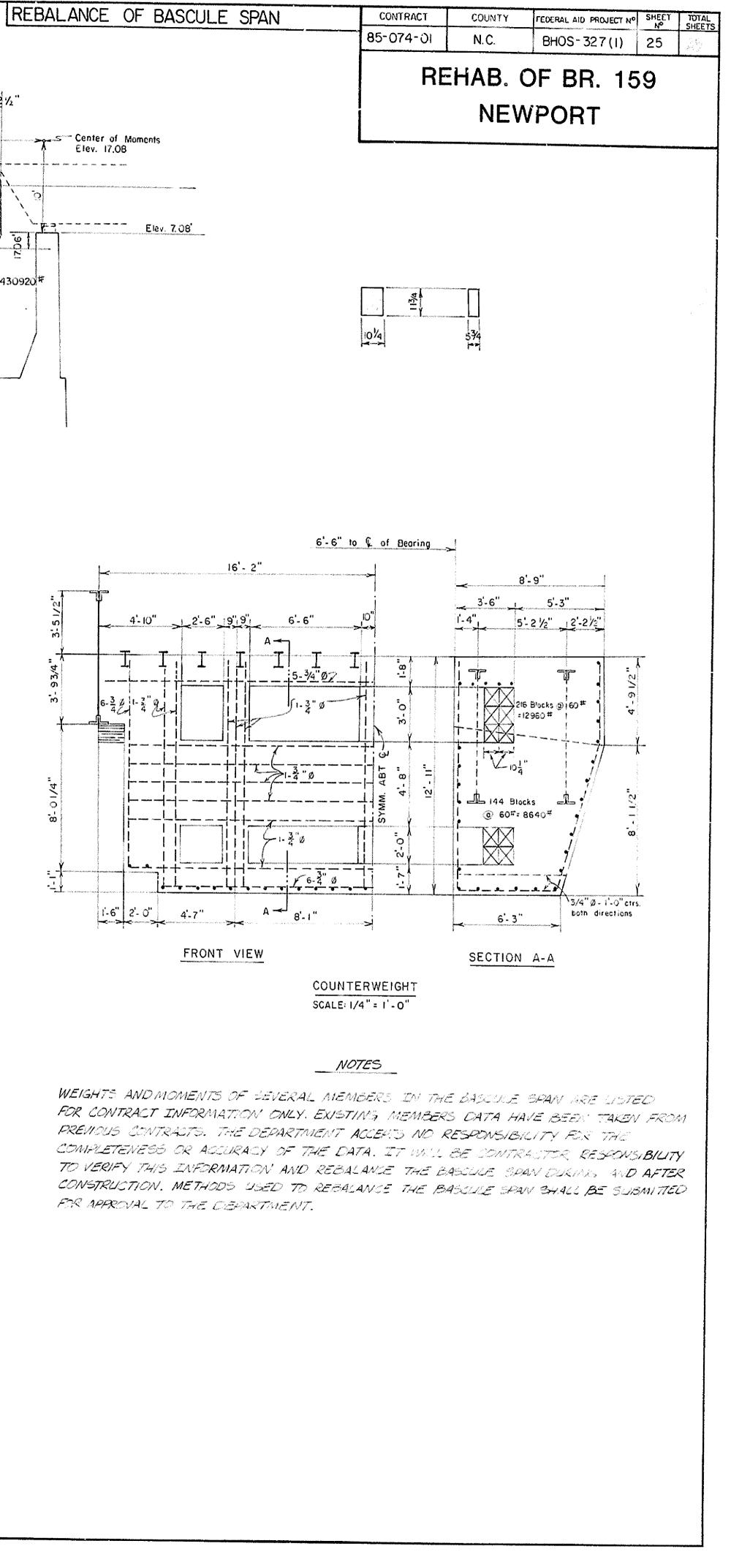
OORBEAM - STRINGERS - CONNECTING ANGLES LOORBEAM - BRACINGS - CONNECTING ANGLES

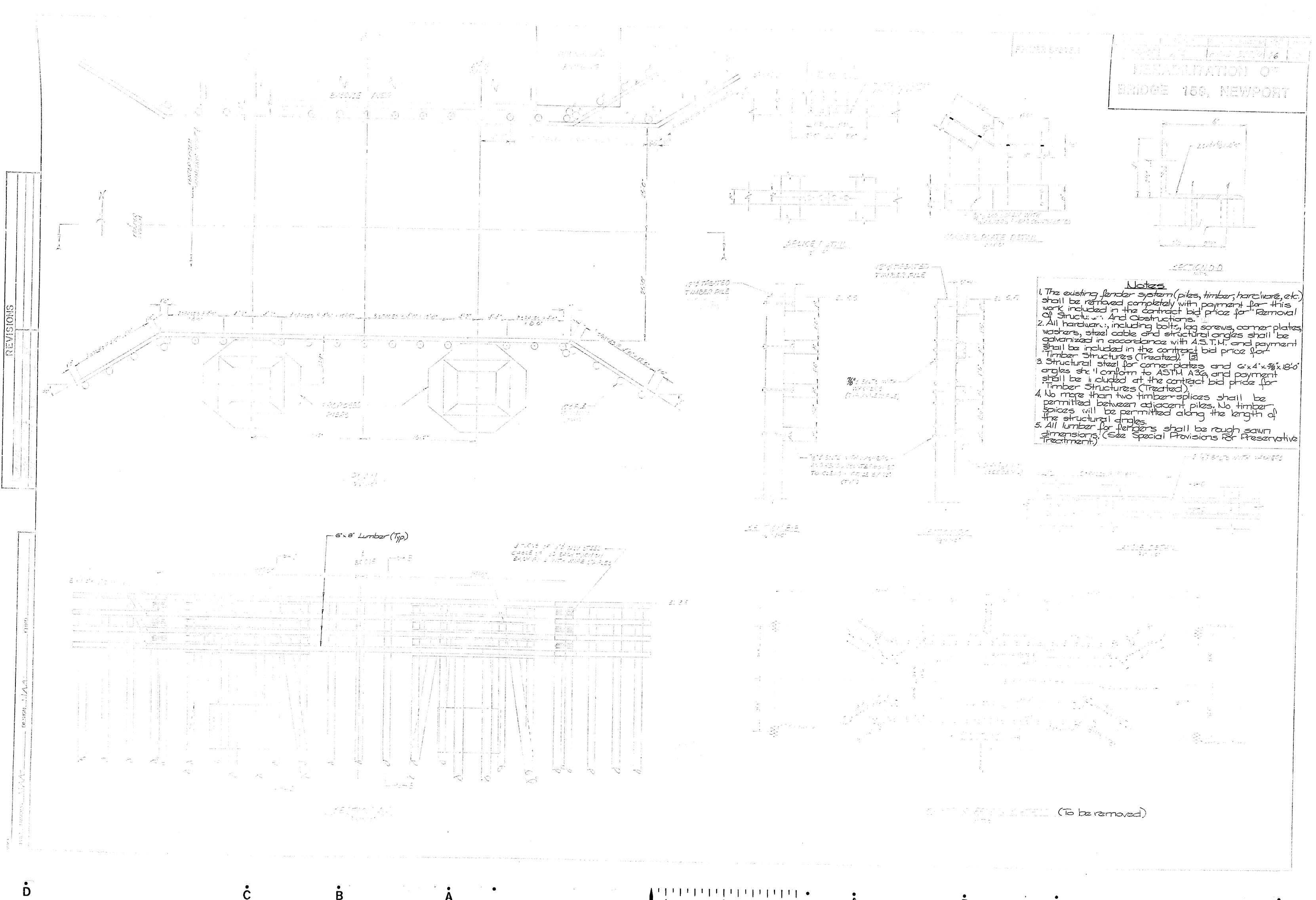
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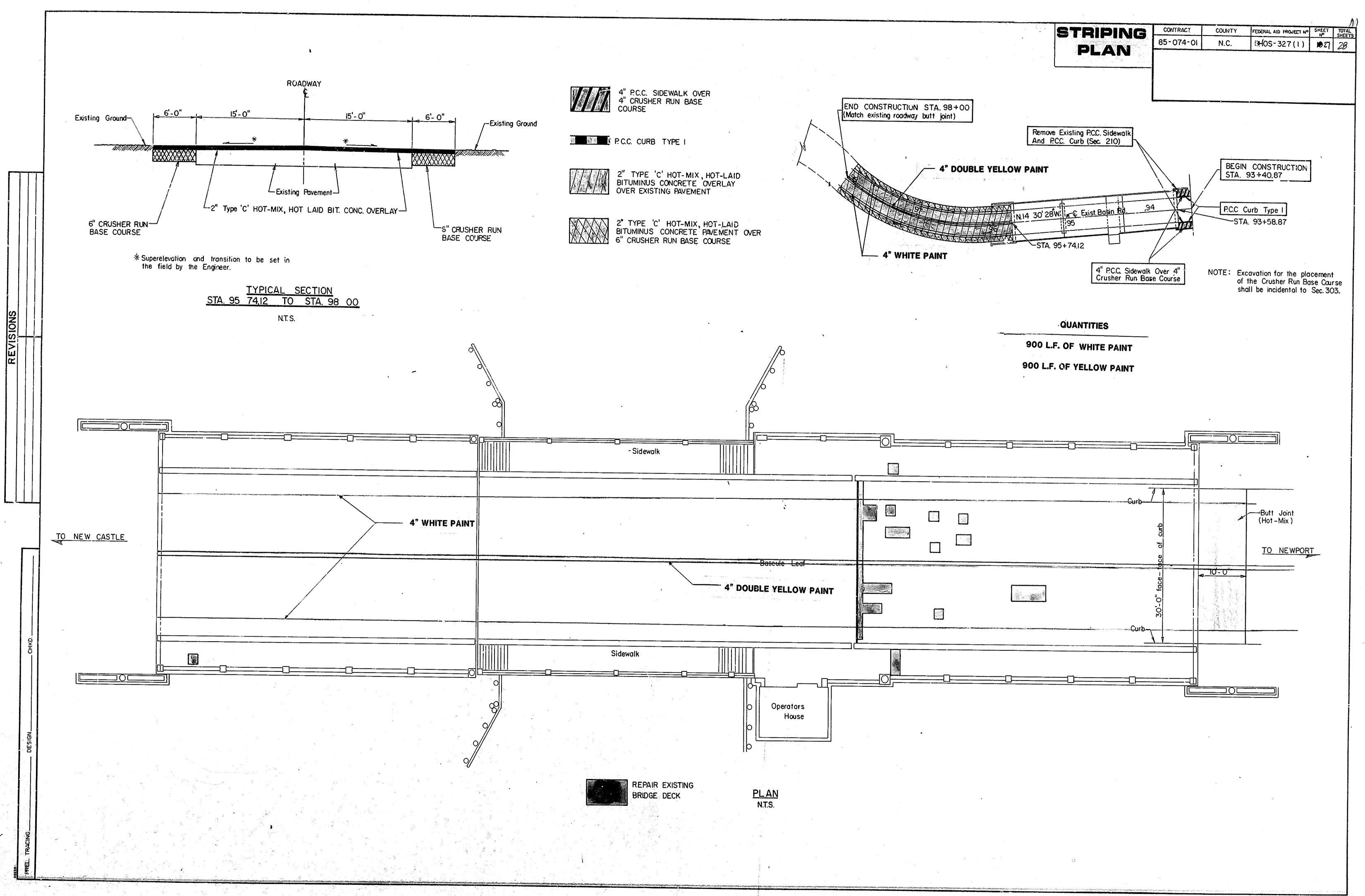
4 Panels 🗿 14'-1" = 56'-4" 6'-3" 7'-10" 15'-21/2" 2'-21/2" <u> 20'</u> 1010 Miles When Hans where they 261832 81 17.6' 10.7 [']

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