

SUBMISSION STATUS: PS&E
DATE PLOTTED: 3/24/2016
JMT FILE LOCATION: S:\MD\101552.011\Standard-Foundations-Foundations\CADD\Structures\SALT BARN 70x100 - FOUNDATIONS\CAD Files\01 - T-1 COVER SHEET.dwg

PROJECT NOTES:

- NO UTILITY RELOCATION INVOLVEMENT IS ANTICIPATED. SHOULD ANY CONFLICTS BE ENCOUNTERED DURING CONSTRUCTION REQUIRING ADJUSTMENT AND/OR RELOCATION OF THE AGENCIES' EXISTING FACILITIES, THE NECESSARY RELOCATION WORK SHALL BE ACCOMPLISHED BY THE RESPECTIVE AGENCIES' FORCES, AS DIRECTED BY THE DISTRICT ENGINEER. ANY ADJUSTMENT AND/OR RELOCATIONS OF MUNICIPALLY OWNED FACILITIES SHALL BE DONE BY THE STATES CONTRACTOR IN ACCORDANCE WITH THE RESPECTIVE AGENCIES STANDARD SPECIFICATIONS AS DIRECTED BY THE DISTRICT ENGINEER.
- NO ENVIRONMENTAL PERMITS ARE REQUIRED FOR THIS WORK PROVIDED NO JURISDICTIONAL WETLANDS OR WATERS ARE IMPACTED. IF THERE IS ANY QUESTION AS TO WHETHER OR NOT A WATER OR WETLAND IS JURISDICTIONAL, CONTACT THE DEL DOT ENVIRONMENTAL SECTION AT 302-760-2264.
- IT IS ANTICIPATED THAT ALL WORK WILL OCCUR WITHIN DELDOT'S EXISTING RIGHT OF WAY OR EASEMENTS AREAS. SHOULD THE NEED OCCUR TO TRESPASS ONTO PRIVATE PROPERTY, IT WILL BE THE RESPONSIBILITY OF THE PROJECT MANAGER TO SECURE SUCH TRESPASS NEEDS.
- IT IS ANTICIPATED THAT ALL WILL OCCUR WITHIN DELDOT'S RIGHT OF WAY. SHOULD THE NEED OCCUR TO TRESPASS ONTO RAILROAD PROPERTY, INCLUDING THE HIGHWAY-RAIL CROSSING, IT WILL BE THE RESPONSIBILITY OF THE PROJECT MANAGER TO CONTACT THE RAILROAD CHIEF ENGINEER AND OBTAIN WRITTEN AUTHORIZATION BEFORE ENTERING.
- THE PROJECT MANAGER SHALL BE RESPONSIBLE FOR COORDINATION WITH THE TRAFFIC SECTION RELATING TO ANY IMPACTS TO TRAFFIC SECTION FACILITIES (INCLUDING BUT NOT LIMITED TO TRAFFIC LOOPS, JUNCTION WELLS ETC.) AT LEAST 4 WEEKS IN ADVANCE OF THE THE START OF THE ACTIVITY. PRIOR TO INITIATING ANY WORK ON THIS CONTRACT (OR SITES), THE PROJECT MANAGER SHALL BE RESPONSIBLE FOR PREPARING AND SUBMITTING FOR APPROVAL OF THE SAFETY SECTION, A MAINTENANCE OF TRAFFIC PLAN. SUFFICIENT TIME SHALL BE PROVIDED FOR THE REVIEW AND APPROVAL OF THE PLAN. THE MAINTENANCE OF TRAFFIC PLAN SHALL INCLUDE PROPOSED TIME RESTRICTIONS ON THE CLOSURE OF TRAVEL LANES SUBJECT TO THE APPROVAL OF THE SAFETY SECTION.
- THE PROJECT MANAGER IS RESPONSIBLE FOR ENSURING ANY REQUIRED DOCUMENTS AND ANALYSIS AS PART OF THE ADOPTED WORK ZONE SAFETY AND MOBILITY PROCEDURES AND GUIDELINES HAS BEEN COMPLETED PRIOR TO ANY WORK STARTING ON THIS CONTRACT.

THE STATE OF DELAWARE DEPARTMENT OF TRANSPORTATION



PROTOTYPICAL DESIGN PLANS FOR: STATEWIDE FABRIC SALT BARN FOUNDATIONS OPEN - END FY17-19

**SALT BARN: 70-FEET BY 100-FEET
AND/OR**

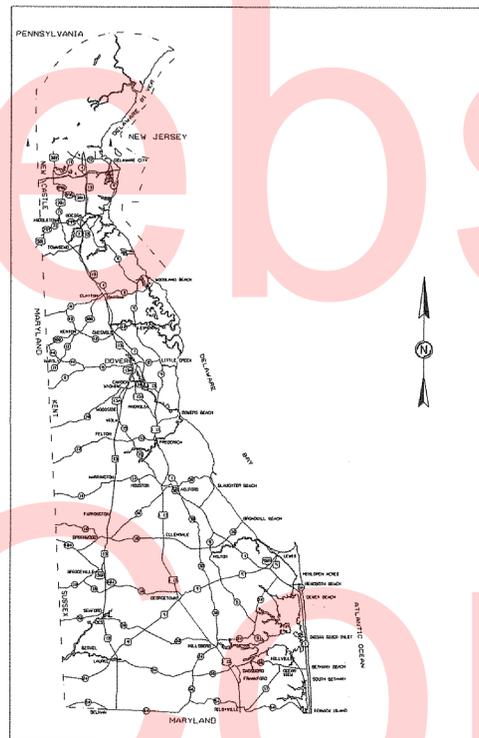
SALT BARN: 80-FEET BY 120-FEET

CONTRACT NUMBER: T201680107

FEDERAL AID PROJECT NUMBER:

COUNTY: _____

M.R. #: _____



LOCATION MAP

SCALE: N. T. S.

U.S. CUSTOMARY
UNITS

DESIGN DESIGNATION

FUNCTIONAL CLASS: N/A	D.H.V. PROJECTED: N/A	YEAR: N/A
TYPE OF CONSTRUCTION: N/A	DESIGN SPEED: N/A	
A.A.D.T. CURRENT: N/A	YEAR: N/A	TRUCKS: N/A
A.A.D.T. PROJECTED: N/A	YEAR: N/A	DIRECTION OF DISTRIBUTION: N/A

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TOTAL SHEETS: 12

APPROVED DESIGN EXCEPTIONS

DESIGN PARAMETER	REQUIRED	PROVIDED	DATE

ADDENDA & REVISIONS

DESCRIPTION	NAME & DATE

ASSOCIATED CONTRACTS

CONTRACT NO.	CONTRACT NAME
T2016800107	STATE WIDE FABRIC SALT BARN OPEN-END FY17-19

T-1

**PREPARED BY
THE CONSULTING FIRM OF**

72 Loveton Circle Baltimore, Maryland 21152-0949
410-329-3100

[Signature] 03/24/2016
RECOMMENDED DATE



RECOMMENDED

[Signature] 4/7/16
STATEWIDE SUPPORT SERVICES ENGINEER DATE

[Signature] 4/7/16
ASSISTANT DIRECTOR STATEWIDE SUPPORT SERVICES DATE

[Signature] 4/7/16
DIRECTOR, MAINTENANCE & OPERATIONS DATE

RECOMMENDED

STORMWATER ENGINEER
DATE _____ SEAL _____

RECOMMENDED AS TO PROCESS

[Signature]
CHIEF ENGINEER
DATE 4/11/2016 SEAL _____

RECOMMENDED

BRIDGE DESIGN ENGINEER
DATE _____ SEAL _____

RECOMMENDED

GROUP ENGINEER, PROJECT DEVELOPMENT
DATE _____ SEAL _____

RECOMMENDED

ASSISTANT DIRECTOR, TRANSPORTATION SOLUTIONS
DATE _____ SEAL _____

RECOMMENDED

CHIEF ENGINEER
DATE _____ SEAL _____

APPROVED

DATE _____ SEAL _____

GENERAL NOTES

GENERAL:

- THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE WORK IS FULLY COMPLETED. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE, AND TO ENSURE THE STABILITY OF THE BUILDING AND ITS COMPONENT PARTS, AND THE ADEQUACY OF TEMPORARY OR INCOMPLETE CONNECTIONS, DURING ERECTION. THIS INCLUDES THE ADDITION OF ANY SHORING, TEMPORARY GUYS, BRACING OR TIEDOWNS THAT MIGHT BE NECESSARY. SUCH MATERIAL IS NOT SHOWN ON THE DRAWINGS. IF APPLIED, THEY SHALL BE REMOVED AS CONDITIONS PERMIT AND SHALL REMAIN THE CONTRACTOR'S PROPERTY.
- THE ENGINEER HAS NO EXPERTISE IN, AND TAKES NO RESPONSIBILITY FOR, CONSTRUCTION MEANS AND METHODS OR JOB SITE SAFETY DURING CONSTRUCTION. PROCESSING AND/OR APPROVING SUBMITTALS MADE BY THE CONTRACTOR WHICH MAY CONTAIN INFORMATION RELATED TO CONSTRUCTION METHODS OR SAFETY ISSUES, OR PARTICIPATION IN MEETINGS WHERE SUCH ISSUES MIGHT BE DISCUSSED, SHALL NOT BE CONSTRUED AS VOLUNTARY ASSUMPTION BY THE ENGINEER OR ANY RESPONSIBILITY FOR SAFETY PROCEDURES. IT IS SOLELY THE RESPONSIBILITY OF EACH CONTRACTOR TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION. THE ENGINEER IS NOT ENGAGED IN, AND DOES NOT SUPERVISE CONSTRUCTION.
- ALL WORK SPECIFIED HEREIN SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST EDITION OF THE IBC BUILDING CODE, THE REFERENCES WITHIN THE IBC BUILDING CODE, AND THE DESIGN CODES AND SPECIFICATIONS, RECOMMEND PRACTICE STANDARDS REFERENCED IN THE CONTRACT DOCUMENTS.

UTILITIES:

- CONTRACTOR IS TO VERIFY LOCATIONS OF ALL UNDERGROUND UTILITIES PRIOR TO PERFORMING ANY SUBSURFACE OR EXCAVATION WORK.
- PROTECTION: PROTECT EXISTING UTILITIES TO REMAIN DURING EXCAVATION, AND CUTTING AND PATCHING, TO PREVENT DAMAGE.
- TAKE ALL PRECAUTIONS NECESSARY TO AVOID CUTTING EXISTING PIPE, CONDUIT OR DUCTWORK SERVING THE SITE. UNTIL PROVISIONS HAVE BEEN MADE TO BYPASS THEM, PROVIDE TEMPORARY SERVICES DURING INTERRUPTIONS TO EXISTING UTILITIES, AS ACCEPTABLE TO OWNER.
- DAMAGES: PROMPTLY REPAIR DAMAGES CAUSED TO ADJACENT FACILITIES BY CONTRACTORS WORK AT THE CONTRACTORS EXPENSE.
- DISPOSAL OF DEMOLISHED MATERIALS: REMOVE DEBRIS, RUBBISH AND OTHER MATERIALS RESULTING FROM EXCAVATION/DEMOLITION OPERATIONS FROM THE BUILDING SITE. TRANSPORT AND LEGALLY DISPOSE OF MATERIALS OFF SITE.
- IF UNANTICIPATED MECHANICAL, ELECTRICAL OR STRUCTURAL ELEMENTS WHICH CONFLICT WITH THE INTENDED FUNCTION OR DESIGN ARE ENCOUNTERED, INVESTIGATE AND MEASURE BOTH NATURE AND EXTENT OF THE CONFLICT. SUBMIT REPORT TO OWNER'S REPRESENTATIVE IN WRITTEN, ACCURATE DETAIL. PENDING RECEIPT OF DIRECTIVE FROM OWNERS' REPRESENTATIVE, REARRANGE SCHEDULE AS NECESSARY TO CONTINUE OVERALL JOB PROGRESS WITHOUT DELAY.

CONTROLLED FILL AND BACKFILL:

- SAMPLES OF ALL MATERIALS THAT THE CONTRACTOR PROPOSES TO USE FOR COMPACTED FILL SHALL BE APPROVED BY THE ENGINEER OF RECORD. COMPACTED FILL SHALL CONSIST OF LOCAL MATERIAL FREE OF DELETERIOUS MATTER AND CLASSIFIED CL, SC, GC, GM, OR SM PER ASTM D-2487. THE CONTROL OF THE MOISTURE FOR PLACING THE FILL WILL BE BASED ON THE RESULTS OF COMPACTION TESTS PER ASTM D-1557. ALL COMPACTED FILL SHALL HAVE A DENSITY OF AT LEAST 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-1557.
- PRIOR TO PLACEMENT OF ANY FILLS, THE SITE SHALL BE STRIPPED OF ALL TOPSOIL, VEGETATION, ROCKS, AND ORGANIC MATERIALS AND THE EXPOSED SUB-GRADE SHALL BE COMPACTED IN PLACE TO A CONFIRMED DENSITY OF 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY. FILL MATERIAL SHALL BE PLACED IN LOOSE LIFTS NOT EXCEEDING 8" IN THICKNESS AND SHALL BE MIXED, SPREAD AND PLACED IN SUCH A WAY AS TO PRODUCE A UNIFORM THICKNESS OF MATERIAL AFTER PLACING.
- EACH LAYER OF FILL SHALL BE COMPACTED WITH A MINIMUM OF 6 COMPLETE PASSES ON ALL PORTIONS OF THE SURFACE OF EACH LIFT OF FILL BY RUBBER-TIRED ROLLERS, SHEEPS-FOOT ROLLERS OR OTHER MECHANICAL EQUIPMENT APPROVED BY THE ENGINEER OF RECORD. COMPACTED FILL PLACED WITHIN 4 FEET OF STRUCTURES AND PIPES SHOULD BE PLACED IN HORIZONTAL LIFTS NOT TO EXCEED 4 INCHES THICKNESS AND COMPACTED WITH HAND TAMPERING LIGHT COMPACTION EQUIPMENT TO THE SAME STANDARD. HEAVY COMPACTION EQUIPMENT IS NOT ALLOWED WITHIN 4 FEET OF STRUCTURES UNLESS A MINIMUM 2 FEET DEPTH OF FILL COVERS THE STRUCTURES. WHENEVER IN PLACE DENSITIES ARE FOUND BELOW ACCEPTABLE LIMITS, ADDITIONAL ROLLING TO PRODUCE THE SPECIFIED DENSITIES SHALL BE REQUIRED.
- THE CONTRACTOR SHALL TAKE ALL MEASURES REQUIRED TO PROVIDE FOR FREE DRAINAGE OF THE SITE AND TO PREVENT PONDING OF WATER. SEDIMENT CONTROL MEASURES SHALL BE MAINTAINED AT ALL TIMES. PLACING OF FILL CONTAINING ORGANIC MATTER; PLACING OF FILL WITH MOISTURE CONTENT TOO HIGH OR TOO LOW FOR PROPER COMPACTION; PLACING OF FILL WHEN FREE WATER IS STANDING ON THE EXISTING FILL SURFACE; PLACING OF FILL IN A FROZEN CONDITION OR ON TOP OF FROZEN MATTER WILL NOT BE PERMITTED.
- DELDOT WILL PERFORM THE FILL AND BACKFILL TESTING AND SHALL SUPERVISE THE PLACING OF THE COMPACTED FILL AND ALL THE MATERIAL AND EQUIPMENT USED FOR ON THIS PROJECT AND SHALL MAKE SUCH SOILS TESTS FOR THE COMPLETION OF THE WORK PERFORMING AT LEAST 6 IN PLACE DENSITY TESTS DURING EACH EIGHT HOUR SHIFT.
- CLASSIFICATION TESTING FOR NATURAL MOISTURE CONTENT GRADATION ANALYSIS AND PROCTOR TESTING SHOULD BE PERFORMED IN THE FIELD DURING CONSTRUCTION TO EVALUATE THE SUITABILITY OF EXCAVATED SOILS FOR REUSE AS FILL AND BACKFILL AS COMPACTED BACKFILL AS APPLICABLE.

SOIL BEARING:

- DESIGN ALLOWABLE BEARING CAPACITY OF 2,000 PSF WAS USED IN THE DESIGN OF THE FOUNDATION SYSTEM. THIS ALLOWABLE BEARING CAPACITY OF 2,000 PSF MUST BE FIELD VERIFY BY A REGISTERED PROFESSIONAL GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF THE FOUNDATION SYSTEM.
- IF SOIL OF THIS BEARING CAPACITY IS NOT ENCOUNTERED AT THE ELEVATIONS INDICATED ON THE CONTRACT DRAWINGS, FOOTINGS SHALL BE LOWERED OR INCREASED IN SIZE AS DIRECTED BY THE ENGINEER OF RECORD.

SUBGRADE PREPARATION:

FOLLOWING THE CLEARING AND STRIPPING OPERATION, THE EXPOSED SUBGRADE SOILS SHALL BE DENSIFIED WITH SUITABLE COMPACTION EQUIPMENT SELECTED AND UNDER THE SUPERVISION OF THE CONTRACTORS GEOTECHNICAL ENGINEER AFTER THE SUBGRADE SOILS HAVE BEEN DENSIFIED, THEY SHALL BE EVALUATED BY THE GEOTECHNICAL ENGINEER FOR STABILITY. ACCORDINGLY, THE SUBGRADE SOILS SHALL BE PROOFROLLED WITH SUITABLE EQUIPMENT TO CHECK FOR AREAS OF SOFT, PUMPING AND/OR YIELDING MATERIAL SHOWING STABILITY FAILURE.

SEVERAL PASSES SHOULD BE MADE WITH SUITABLE EQUIPMENT OVER THE BUILDING PAVING AND FOUNDATION AREA. THE NUMBER OF PASSES SHOULD BE DETERMINED BY THE CONTRACTORS GEOTECHNICAL ENGINEER DEPENDING ON SOIL CONDITIONS.

ANY PUMPING, AND/OR YIELDING AND/OR UNSTABLE SOILS SHOWING FAILURE DURING PROOFROLLING BEYOND THE INITIAL CLEARING OUT SHALL BE UNDERCUT AND/OR STABILIZED AS DIRECTED BY THE GEOTECHNICAL ENGINEER.

FOUNDATIONS-SPREAD FOOTINGS:

- ALL FOUNDATIONS SHALL BEAR ON UNDISTURBED SOIL WITH THE ALLOWABLE BEARING CAPACITY SPECIFIED.
- BOTTOM OF ALL FOOTINGS SHALL BE A MINIMUM OF 3'-0" BELOW ORIGINAL GRADE OR PLACED IN APPROVED COMPACTED FILL.
- ELEVATIONS SHOWN ON PLAN ARE TO THE BOTTOM OF FOOTINGS.

- CONCRETE SHALL NOT BE POURED ON FROZEN GROUND/OR IN WET CONDITIONS.
- PROVIDE SHORING AS REQUIRED TO SUPPORT LATERAL LOADS DURING EXCAVATION.
- FILL ALL VOIDS AND REPLACE DISTURBED SOIL WITH LEAN CONCRETE

FOUNDATIONS AND SLAB-ON-GRADE:

BOTTOM OF ALL FOOTINGS SHALL BE A MINIMUM OF 3'-0" BELOW ORIGINAL GRADE OR PLACED IN APPROVED COMPACTED FILL. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 3'-0" BELOW FINISHED GRADE. A SOIL BEARING CAPACITY AND MODIFIED SUBGRADE REACTION AS INDICATED BELOW WAS USED IN THE FOUNDATION AND PAVEMENT DESIGN, AND MUST BE FIELD VERIFIED BY A REGISTERED GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF THE FOUNDATIONS AND PAVEMENT. IF SOIL OF THIS BEARING CAPACITY OR MODULUS OF SUBGRADE REACTION IS NOT ENCOUNTERED AT THE ELEVATIONS INDICATED ON THE CONTRACT DRAWINGS, FOOTINGS SHALL BE LOWERED OR INCREASED IN SIZE OR THE SUBGRADE PREPARED AS DIRECTED BY THE ENGINEER OF RECORD. ELEVATIONS SHOWN ON PLAN ARE TO THE BOTTOM OF THE FOOTINGS.

CALCULATIONS FOR TOTAL BUILDING SETTLEMENTS OR DIFFERENTIAL BUILDING SETTLEMENTS HAVE NOT BEEN PERFORMED.

BEARING CAPACITY AND MODULUS OF SUBGRADE REACTION:

BUILDING: 2,000 PSF
MODULUS OF SUBGRADE REACTION (K)
FOR PAVEMENT STRUCTURE: 150 POUNDS PER CUBIC INCH

THE ELEVATION FOR THE BOTTOM OF THE FOUNDATIONS HAS BEEN UNIFORMLY SET BELOW FINISHED GRADE AS INDICATED. IF THE FINISHED GRADE VARIES MORE THAN 6-INCHES ACROSS THE WIDTH OR LENGTH OF THE BUILDING, THEN THE FOUNDATIONS SHALL BE STEPPED AS DIRECTED BY THE ENGINEER, (REFERENCE 5/S-06, TYPICAL STEPPED FOOTING).

THE FOUNDATIONS, WALLS, AND PIERS HAVE BEEN DESIGNED FOR THE ASSUMED "PRELIMINARY REACTIONS" INDICATED. THESE REACTIONS SHALL BE VERIFIED BY THE ENGINEER AGAINST THE ACTUAL SALT BARN REACTIONS DATA SUPPLIED BY THE SALT BARN MANUFACTURER. SHOULD THE ACTUAL REACTION DATA EXCEED THE ASSUMED PRELIMINARY REACTIONS, MODIFICATIONS TO THE FOUNDATIONS, WALLS, AND PIERS MAY BE REQUIRED AS DETERMINED BY THE ENGINEER.

THE OWNER SHALL PROVIDE ACTUAL SALT BARN REACTION DATA PRIOR TO SUBMITTAL OF THE REINFORCING STEEL SHOP DRAWINGS SO AS TO ALLOW FOR MODIFICATIONS IF REQUIRED SO AS NOT TO DELAY THE PROJECT.

CONCRETE:

- ALL CONCRETE WORK SHALL CONFORM TO ALL THE PROVISIONS OF THE "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301) AND TO THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318).
- ALL STRUCTURAL CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH $f'_c = 4,000$ psi AND NOT EXCEED A WATER CEMENT RATIO W/C OF 0.45, UNLESS NOTED OTHERWISE.
- ALL CONCRETE SHALL CONFORM TO ALL THE PROVISIONS OF "RECOMMENDED PRACTICE FOR HOT WEATHER CONCRETING" (ACI 305R-10) AND "RECOMMENDED PRACTICE FOR COLD WEATHER CONCRETING" (ACI 306R-10).
- ALL FORMWORK SHALL BE IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTE "FORMWORK FOR CONCRETE" SPECIAL PUBLICATION NO. 4 AND ACI'S "STANDARD RECOMMENDED PRACTICE FOR CONCRETE FORMWORK" (ACI-347-LATEST EDITION).
- ALL CONCRETE SHALL HAVE AN AIR ENTRAINMENT OF 5% $\pm 1\%$. NO ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL BE PERMITTED.
- THE MAXIMUM SLUMP OF ALL CONCRETE SHALL BE 4".
- PROVIDE 3/4" CHAMFER AT EXPOSED CORNERS, INCLUDING TOP OF RETAINING WALLS.
- SLABS SHALL BE FINISHED TO A MINIMUM FLATNESS F- NUMBER $F_f = 20$ AND A MINIMUM LEVELNESS $F-NUMBER F_1 = 17$ IN ANY DIRECTION.
- ALL CONCRETE SHALL BE CURED WITH LIQUID SEALING COMPOUND CONFORMING TO ASTM C-309, TYPE I AND FEDERAL SPECIFICATION TT-C-00800 OR OTHER APPROVED METHOD WHICH IS COMPATIBLE WITH FLOORING ADHESIVES AND OTHER SURFACE TREATMENTS. ALL CONCRETE LEFT EXPOSED AT THE COMPLETION OF THE PROJECT SHALL BE TREATED WITH A CLEAR, PENETRATING ACRYLIC BASE POLYMER CAPABLE OF PREVENTING INFILTRATION OF WATER BORNE CHLORIDES.
- LOADS GREATER THAN THE DESIGN LOADS SHALL NOT BE PLACED ON THE STRUCTURE. CONCRETE MAY NOT SUPPORT ITS DESIGN LOAD FOR 28 DAYS. ONE SET OF COMPRESSIVE TEST CYLINDERS FOR EACH 100 CUBIC YARDS POURED, BUT NOT LESS THAN ONE SET FOR EACH DAY'S POUR AND EACH CLASS OF CONCRETE, ALONG WITH SLUMP TESTS SHALL BE PERFORMED BY A QUALIFIED TESTING LABORATORY APPROVED BY THE ENGINEER OF RECORD.

REINFORCING STEEL:

- REINFORCING STEEL SHALL BE NEW DEFORMED BARS IN ACCORDANCE WITH ASTM A-615, GRADE 60. BENDS ARE TO BE FABRICATED AS PER CRSI MANUAL OF STANDARD PRACTICE.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185, GRADE 60.
- PLACE MAIN REINFORCING STEEL SO AS TO PROVIDE 3" MINIMUM COVER FOR FOUNDATIONS POURED ON EARTH, 2" MINIMUM COVER FOR INTERIOR AND EXTERIOR FACE OF WALLS AND SLABS, (EXCEPT AS OTHERWISE DETAILED).
- ALL WALL AND SLAB STEEL SHALL HAVE A MINIMUM EXTENSION INTO THE SUPPORTS IN ACCORDANCE WITH THE LATEST ADDITION OF THE ACI CODE, PROVIDE ACCESSORIES AND BAR SUPPORTS IN ACCORDANCE WITH THE MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (ACI 315).
- UNLESS OTHERWISE NOTED, WELDED WIRE FABRIC (WWF) REINFORCING SHALL BE PLACED AT MID-DEPTH OF SLABS ON GRADE AND DRAPED OVER SUPPORTS IN CONCRETE SLABS ON CENTERING. END LAPS OF ALL WWF REINFORCING SHALL BE LAPPED 8" MINIMUM.
- WHERE INDICATED EPOXY COATED REINFORCING SHALL COMPLY WITH ASTM A775. PROVIDE EPOXY COATED BAR SUPPORTS AT EPOXY COATED REINFORCING.
- WHERE INDICATED EPOXY COATED WELDED WIRE FABRIC SHALL COMPLY WITH ASTM A884, CLASS A, TYPE 1 COATED. PROVIDE EPOXY COATED SUPPORTS AT EPOXY COATED WELDED WIRE FABRIC.

STEEL:

- STRUCTURAL STEEL PLATES, ANGLES, AND BARS, SHALL CONFORM TO ASTM A-36, GR 36.
- WELDS SHALL CONFORM TO ALL THE PROVISIONS OF THE STRUCTURAL WELDING CODE, AWS D1.1-2015 OF THE AMERICAN WELDING SOCIETY.
- HEADED STUD TYPE SHEAR CONNECTORS SHALL BE COLD FINISHED CARBON STEEL WELDABLE COMPLYING WITH ASTM A-108, GRADE 1015 OR 1020, WITH DIMENSIONS COMPLYING WITH AISC SPECIFICATIONS.
- ALL STEEL SHALL BE HOT-DIP GALVANIZED WITH AN APPLIED ZINC RICH COATING BY THE HOT-DIP PROCESS ACCORDING TO ASTM A123 AFTER FABRICATION.
 - APPLY KEM BOND HS UNIVERSAL PRIMER, PRIOR TO INTERMEDIATE AND TOP COATS BY SHERWIN WILLIAMS, MPI #79.
 - APPLY THE INDUSTRIAL ENAMEL HS HIGH SOLIDS, LOW VOC, ALKYD, GLOSS AS AN INTERMEDIATE TOP COAT, MPI #48.

SHOP DRAWINGS:

COMPREHENSIVE AND COMPLETE SHOP DRAWINGS FOR ALL STRUCTURAL SYSTEMS, ELEMENTS AND THE SALT BARN SHOWN ON THE CONTRACT DOCUMENTS MUST BE SUBMITTED TO THE GENERAL CONTRACTOR AND REVIEWED BY THE ENGINEER OF RECORD. ALL CONTRACTOR MODIFICATIONS (INCLUDING PRODUCT SUBMISSIONS) MUST BE IDENTIFIED IN WRITING AS A PROPOSED "AS EQUAL" CHANGE AT TIME OF SUBMISSION. IF A CONTRACTOR OR OWNER FAILS TO SUBMIT THE SHOP DRAWINGS AND SALT BARN REACTION DATA, OR FAILS TO FOLLOW THE ABOVE "AS EQUAL" PROCEDURE, JMT WILL NOT BE RESPONSIBLE FOR THE STRUCTURAL CERTIFICATION AND DESIGN OF THE PROJECT. SHOP DRAWINGS ARE REVIEWED BY THE ENGINEER AS A CONVENIENCE TO THE CONTRACTOR AND ARE NOT A CONTRACT DOCUMENT.

INSPECTION:

ALL WORK SPECIFIED HEREIN SHALL BE INSPECTED IN ACCORDANCE WITH THE BUILDING CODE AND ALL LOCAL ORDINANCES. DELDOT WILL HIRE AN EXPERIENCED QUALIFIED INSPECTOR TO PERFORM ALL REQUIRED INSPECTION WORK. INSPECTION SHALL CONSIST OF, BUT NOT BE LIMITED TO, VISUAL OBSERVATIONS OF MATERIALS, EQUIPMENT OR CONSTRUCTION WORK FOR THE PURPOSE OF ASCERTAINING THAT THE WORK IS IN SUBSTANTIAL CONFORMANCE WITH THE CONTRACT DOCUMENTS AND WITH THE DESIGN INTENT. THE ENGINEER WILL NOT PERFORM THE REQUIRED INSPECTION. THE ENGINEER MAY VISIT THE SITE TO ASCERTAIN GENERAL CONFORMANCE TO THE CONTRACT DOCUMENTS. HOWEVER, SUCH VISITS SHALL NOT BE RELIED UPON BY OTHERS AS ACCEPTANCE OF THE WORK, NOR SHOULD IT BE CONSTRUED TO RELIEVE THE CONTRACTOR IN ANY WAY FROM HIS OBLIGATIONS AND RESPONSIBILITIES UNDER THE CONSTRUCTION CONTRACT.

DESIGN CODES AND SPECIFICATIONS:

- AMERICAN SOCIETY OF CIVIL ENGINEERS, STRUCTURAL ENGINEERING INSTITUTE, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, (ASCE/SEI 7-10)
- 2012 INTERNATIONAL BUILDING CODE (IBC)
- ASCE STANDARD 17-96, AIR SUPPORTED STRUCTURES
- ASCE STANDARD 19-10, STRUCTURAL APPLICATIONS OF STEEL CABLES FOR BUILDINGS
- ASCE STANDARD 55-10, TENSILE MEMBRANE STRUCTURES
- ASCE 2013, TENSILE FABRIC STRUCTURES, DESIGN, ANALYSIS, AND CONSTRUCTION
- AMERICAN CONCRETE INSTITUTE (ACI)
- AMERICAN INSTITUTE OF STEEL CONSTRUCTION 14TH EDITION (AISC)
- AMERICAN IRON AND STEEL INSTITUTE (AISI)

DESIGN DATA:

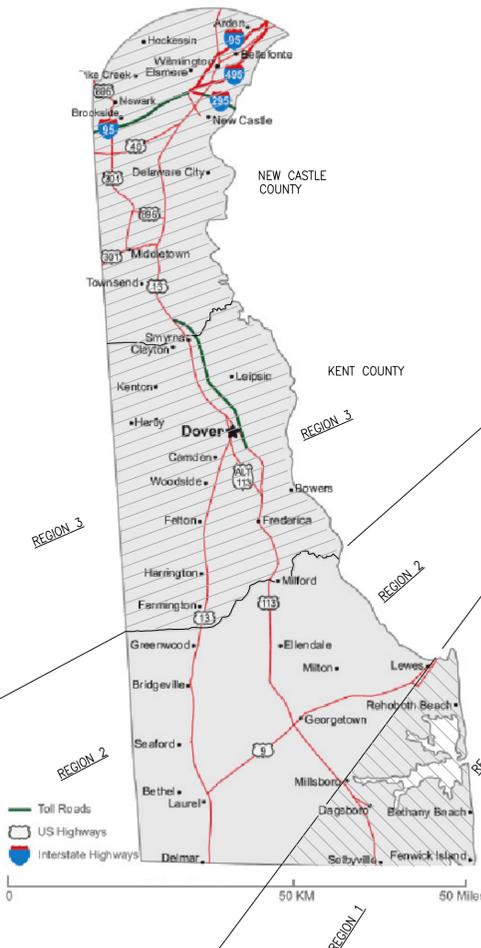
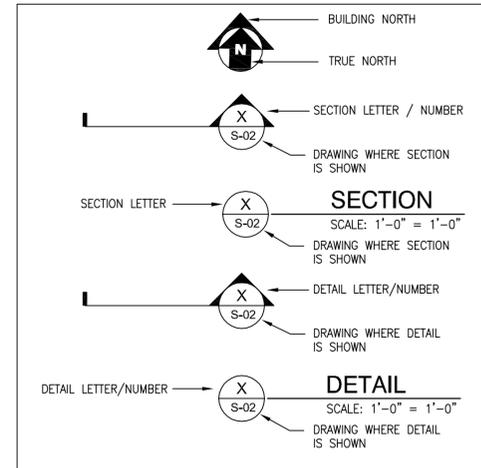
$f'_c = 4,000$ psi (FOOTINGS, SLABS AND WALLS)
 $f_y = 60,000$ psi (CONCRETE REINFORCING)
 $F_y = 36,000$ psi/50,000 psi

RISK CATEGORY:

I

CRITERIA & LOADING	REGIONS		
	1. Coastal Regions to Lewes-Millsboro	2. North of Lewes-Millsboro to Milford	3. Kent and New Castle Counties
Minimum Design Loads:			
A. Building Classification Category	I	I	I
B. Ground Snow, P_g	20 PSF	25 PSF	25 PSF
Flat Roof Snow Load, P_f	12.1 PSF	15.1 PSF	15.1 PSF
Snow Exposure Factor, C_e	0.9	0.9	0.9
Snow Thermal Factor, C_t	1.2	1.2	1.2
Snow Importance Factor, I_s	0.80	0.80	0.80
C. Wind Speed	116 MPH	110 MPH	105 MPH
Exposure	C	C	C
Importance Factor, I_w	1.00	1.00	1.00
Internal Pressure Coefficient, GC_{pi}	± 0.18	± 0.18	± 0.18
Enclosed Building			
D. Temperature Range	-20' F - 110' F	-20' F - 110' F	-20' F - 110' F
E. Maximum Collateral Load	1.0 PSF	1.0 PSF	1.0 PSF
Delegated to Salt Barn Manufacturer			
F. Roof Live Load	ASCE 7-10	ASCE 7-10	ASCE 7-10
G. Seismic Design	ASCE 7-10	ASCE 7-10	ASCE 7-10
Seismic Importance Factor, I_p	1.00	1.00	1.00
Ss Mapped Spectral Response Acceleration	0.100g	0.111g	0.187g
S1 Mapped Spectral Response Acceleration	0.045g	0.047g	0.058g
Site Soil Class (To be Confirmed by Geotechnical Report)	D	D	D
Spectral Coefficient, S_{DS}	0.107g	0.118g	0.200g
Spectral Coefficient, S_{D1}	0.072g	0.075g	0.093g
Seismic Design Category	B	B	B
Basic Structural System	Moment-resisting frame system	Moment-resisting frame system	Moment-resisting frame system
Seismic Force Resisting System	Steel System Not Specifically detailed for Seismic Resistance (Table 12.2-1H, ASCE 7-10)	Steel System Not Specifically detailed for Seismic Resistance (Table 12.2-1H, ASCE 7-10)	Steel System Not Specifically detailed for Seismic Resistance (Table 12.2-1H, ASCE 7-10)
H. Assumed Minimum Soil Bearing Pressure (To be confirmed by Geotechnical Report)	2000 PSF	2000 PSF	2000 PSF
I. Ice	Chapter 10 ASCE 7-10	Chapter 10 ASCE 7-10	Chapter 10 ASCE 7-10
Ice Importance Factor, I_i	0.80	0.80	0.80
J. The maximum allowable deflection of any structural member shall be $L/240$, and the maximum story drift shall be $H/50$ for seismic and $H/400$ for wind.			

LEGEND



REGION MAP



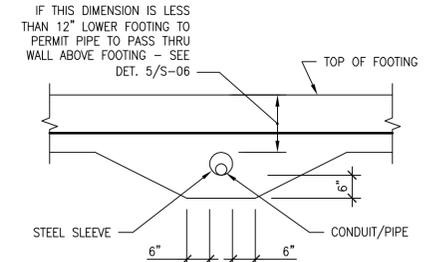
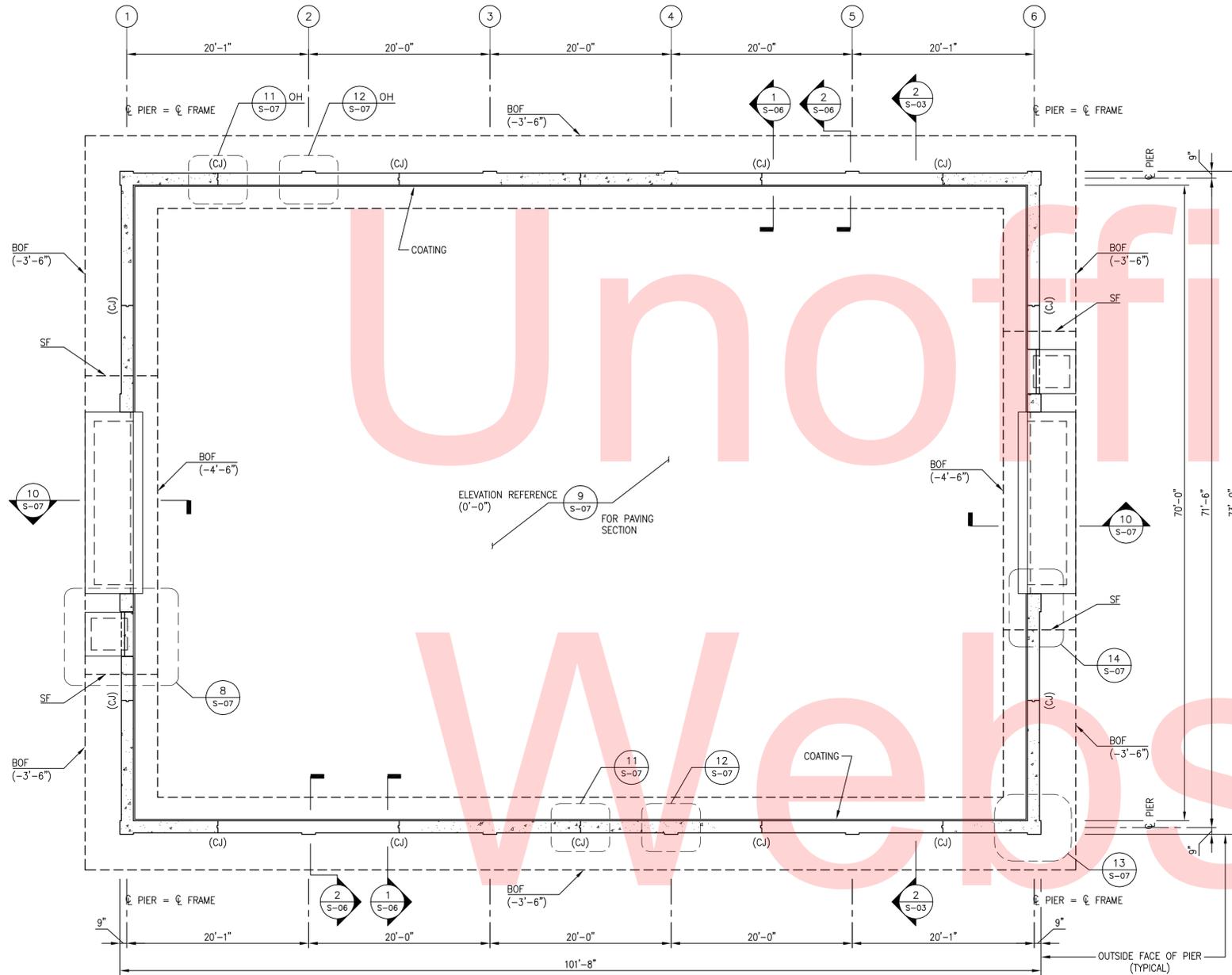
S-01

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CONTRACT	DATE	03-18-2016
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COUNTY	CHECKED BY:	JHB
STATEWIDE		

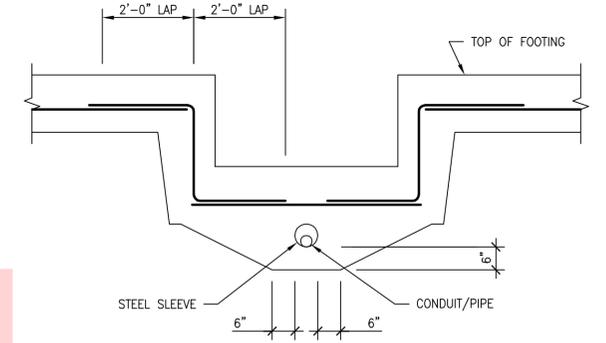
SHEET NO.	2
TOTAL SHTS.	8

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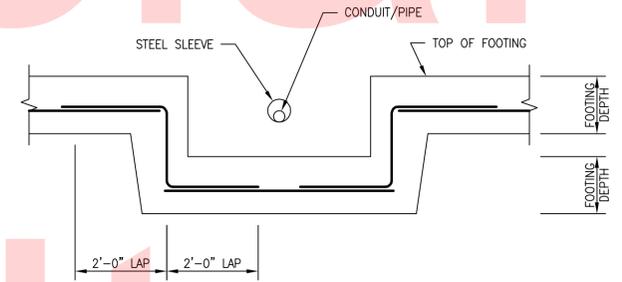
• WHEN PIPE IS MORE THAN 12" BELOW BOTTOM OF FOOTING SHOWN BY ELEVATIONS ON PLAN, STEP FOOTING DOWN TO PERMIT PIPE TO PASS DIRECTLY BELOW FOOTING - SEE DET. 3 ON THIS SHEET

2
TYP. CONDUIT/PIPE THRU FOOTING
SCALE: N.T.S.



• STEP FOOTING DOWN AS REQUIRED IN ACCORDANCE WITH DET. 5/S-06

3
TYP. FOOTING STEPPED DOWN AT CONDUIT/PIPE
SCALE: N.T.S.



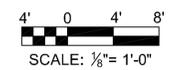
• STEP FOOTING DOWN AS REQUIRED IN ACCORDANCE WITH DET. 5/S-06

4
TYP. FOOTING STEPPED DOWN AT CONDUIT/PIPE
SCALE: N.T.S.

1
S-02
SALT BARN - WALL & FOUNDATION PLAN
SCALE: 1/8" = 1'-0"

NOTES:

- SALT STORAGE 70' x 100'
 - ASSUMED UNIT WEIGHT OF SALT MATERIAL: 100 POUNDS PER CUBIC FOOT.
 - ASSUMED ANGLE OF REPOSE: 34 DEGREES FROM HORIZONTAL.
 - APPROXIMATE ALLOWABLE VOLUME OF SALT MATERIAL EVENLY DISTRIBUTED: 3,780 CUBIC YARDS.
 - APPROXIMATE ALLOWABLE WEIGHT OF SALT MATERIAL EVENLY DISTRIBUTED: 5,100 TONS.
 - APPROXIMATE SOIL PRESSURE DUE TO SALT MATERIALS: 1,457 PSF.
- REFERENCE ELECTRICAL DRAWINGS FOR LOCATION OF UTILITIES, ENTERING THE BUILDING, COORDINATE LOCATION AND ELEVATION WITH DETAILS 2/S-02, 3/S-02 AND 4/S-02 THIS SHEET.
- ELEVATION BOTTOM OF FOOTINGS (BOF) ARE SHOWN ON PLAN THUS: $BOF(-X'-X")$ AND REFERENCED FROM TOP OF PAVING SECTION AT REFERENCE ELEVATION 0'-0", AND WITH A MAXIMUM DIFFERENCE FROM THE TOP OF THE PAVING SECTION TO FINISH GRADE OF 6-INCHES.
- STEPS IN THE FOOTINGS ARE SHOWN ON PLAN THUS: $(-)$ REFERENCE 5/S-06 FOR TYPICAL STEPPED FOOTING DETAIL.
- THE ELEVATION FOR THE BOTTOM OF THE FOUNDATIONS HAS BEEN UNIFORMLY SET BELOW FINISHED GRADE AS INDICATED. IF THE FINISHED GRADE VARIES MORE THAN 6-INCHES ACROSS THE WIDTH OR LENGTH OF THE BUILDING, THEN THE FOUNDATIONS SHALL BE STEPPED AS DIRECTED BY THE ENGINEER, (REFERENCE 5/S-06, TYPICAL STEPPED FOOTING).



S-02



ADDENDUMS / REVISIONS	

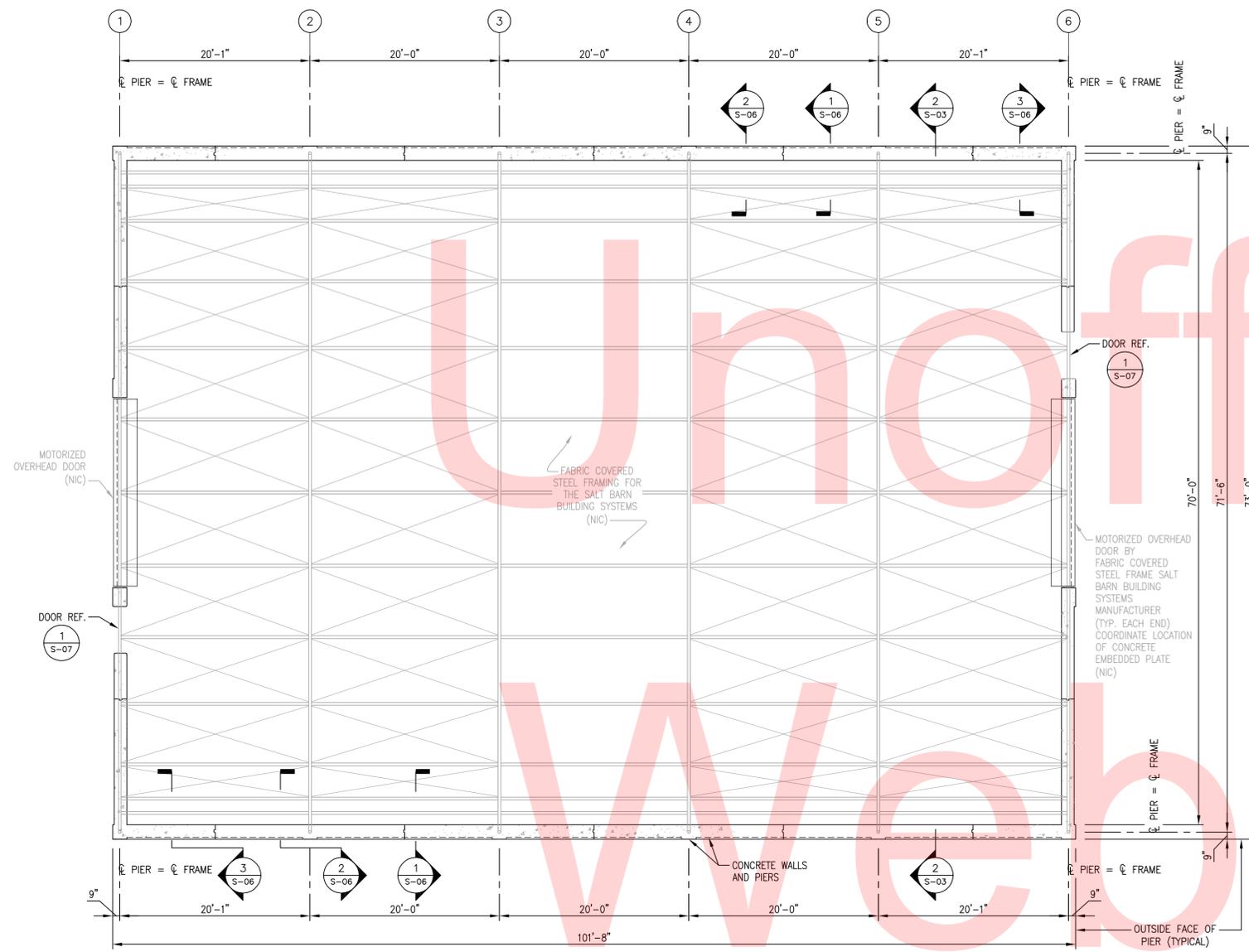
STATEWIDE FABRIC SALT BARN
FOUNDATIONS
OPEN - END FY 17-19

CONTRACT	DATE	03-18-2016
T201680107	DESIGNED BY:	NLL / JZ / DL
COUNTY	CHECKED BY:	JHB
STATEWIDE		

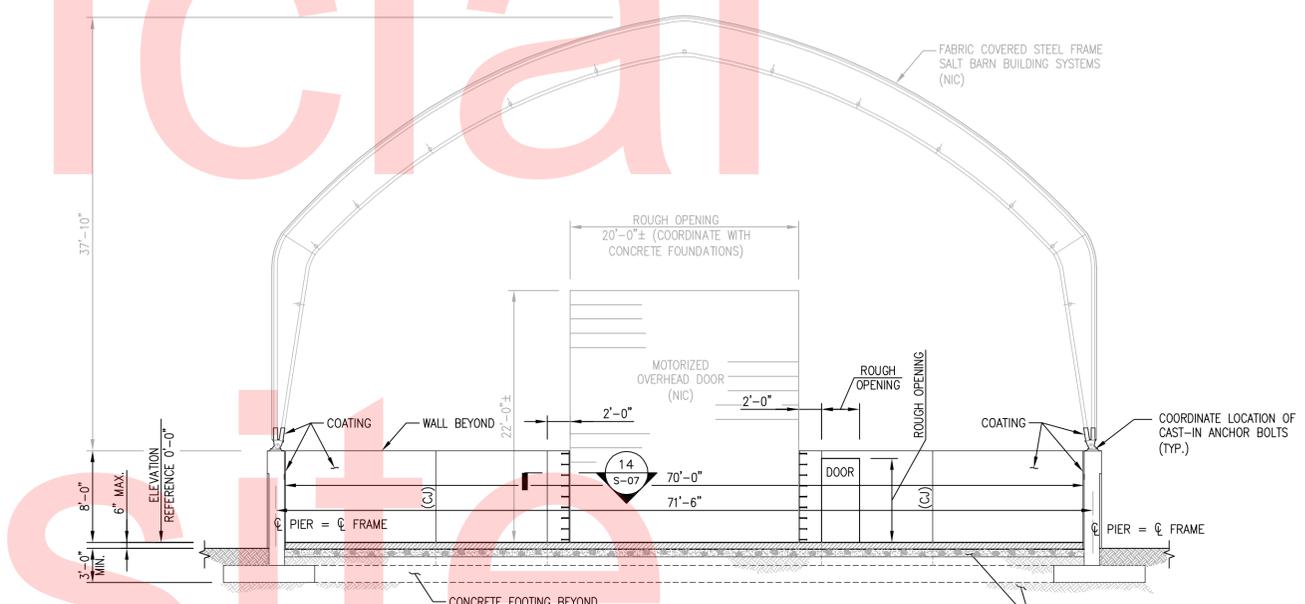
SALT BARN 70' x 100'
WALL & FOUNDATION PLAN

SHEET NO.	3
TOTAL SHTS.	8

- NOTES:
1. A REPRESENTATIVE OF THE FABRIC COVERED STEEL FRAME SALT BARN BUILDING SYSTEMS MANUFACTURER SHALL BE ON SITE TO SUPERVISE THE PLACEMENT OF THEIR CAST-IN ANCHOR BOLTS.
 2. GRAYED OR GHOSTED LINES INDICATES WORK PERFORMED BY OTHERS AND IS PROVIDED FOR REFERENCE PURPOSES.

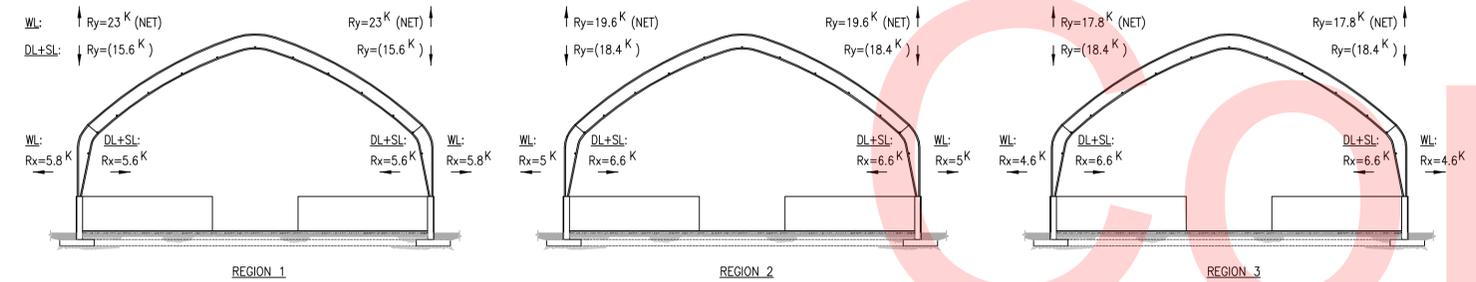


1 SALT BARN - FRAMING PLAN
SCALE: 1/8" = 1'-0"



2 SALT BARN - FRAMING SECTION
SCALE: 1/8" = 1'-0"

WL: WIND LOAD
SL: SNOW LOAD
DL: DEAD LOAD



ASSUMED PRELIMINARY REACTIONS - 70' x 100'
SCALE: N.T.S.

- NOTES:
1. REFERENCE S-01 "REGION MAP" FOR REGION LOCATION, CRITERIA & LOADINGS.



4' 0 4' 8'
SCALE: 1/8" = 1'-0"

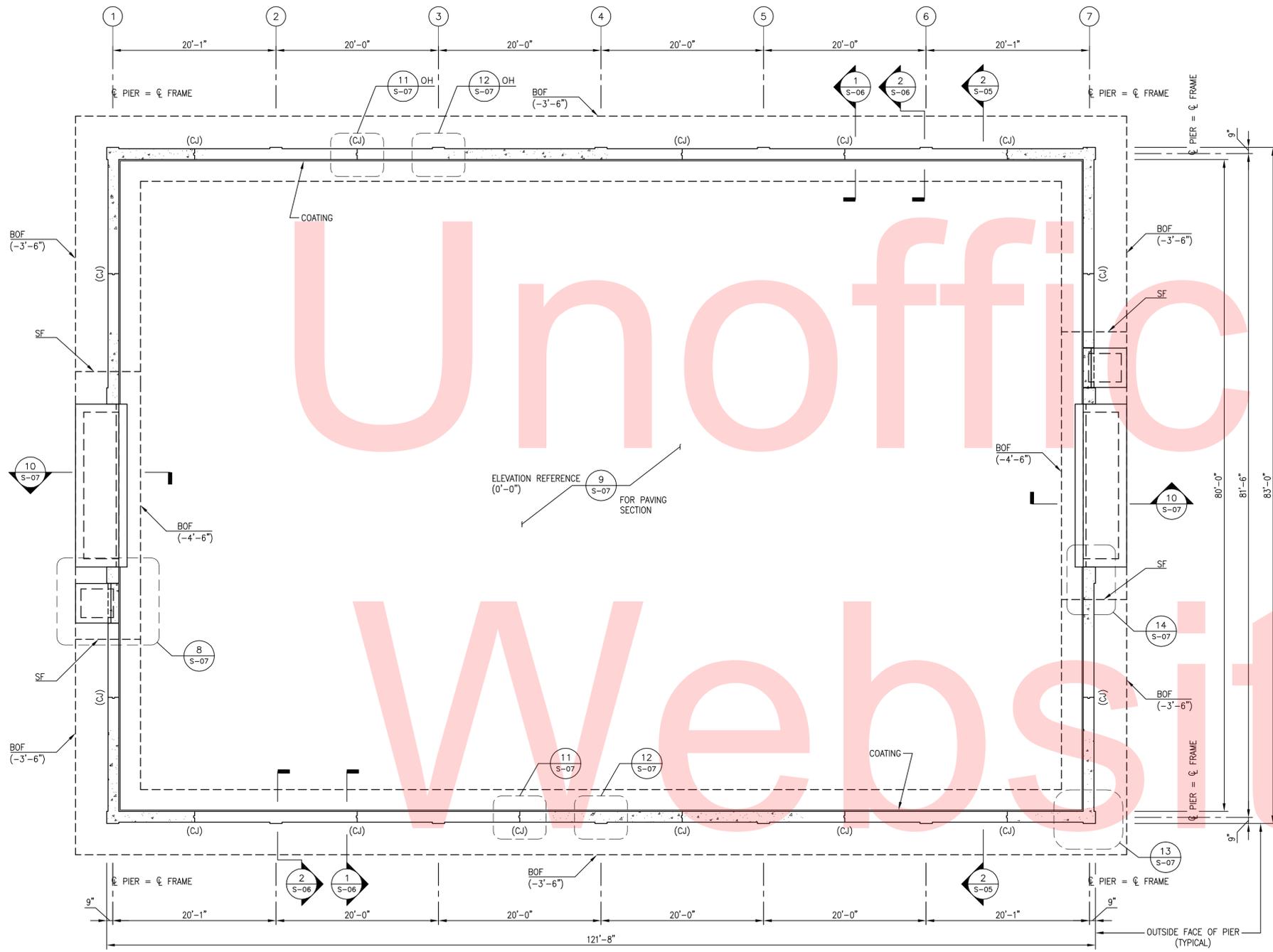
S-03

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ADDENDUMS / REVISIONS	

CONTRACT	DATE	03-18-2016
T201680107	DESIGNED BY:	NLL / JZ / DL
COUNTY	CHECKED BY:	JHB
STATEWIDE		

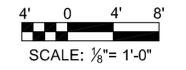
SHEET NO.	4
TOTAL SHTS.	8



1 S-04 SALT BARN - WALL & FOUNDATION PLAN
SCALE: 1/8" = 1'-0"

NOTES:

1. SALT STORAGE 80' x 120'
 - A. ASSUMED UNIT WEIGHT OF SALT MATERIAL: 100 POUNDS PER CUBIC FOOT.
 - B. ASSUMED ANGLE OF REPOSE: 34 DEGREES FROM HORIZONTAL.
 - C. APPROXIMATE ALLOWABLE VOLUME OF SALT MATERIAL EVENLY DISTRIBUTED: 5,184 CUBIC YARDS.
 - D. APPROXIMATE ALLOWABLE WEIGHT OF SALT MATERIAL EVENLY DISTRIBUTED: 7,000 TONS.
 - E. APPROXIMATE SOIL PRESSURE DUE TO SALT MATERIALS: 1,458 PSF.
2. REFERENCE ELECTRICAL DRAWINGS FOR LOCATION OF UTILITIES, ENTERING THE BUILDING. COORDINATE LOCATION AND ELEVATION WITH DETAILS 2/S-02, 3/S-02 AND 4/S-02.
3. ELEVATION BOTTOM OF FOOTINGS (BOF) ARE SHOWN ON PLAN THUS: $(-X'-X'')$ AND REFERENCED FROM TOP OF PAVING SECTION AT REFERENCE ELEVATION 0'-0", AND WITH A MAXIMUM DIFFERENCE FROM THE TOP OF THE PAVING SECTION TO FINISH GRADE OF 6-INCHES.
4. STEPS IN THE FOOTINGS ARE SHOWN ON PLAN THUS: $(- - -)$ REFERENCE 5/S-06 FOR TYPICAL STEPPED FOOTING DETAIL.
5. THE ELEVATION FOR THE BOTTOM OF THE FOUNDATIONS HAS BEEN UNIFORMLY SET BELOW FINISHED GRADE AS INDICATED. IF THE FINISHED GRADE VARIES MORE THAN 6-INCHES ACROSS THE WIDTH OR LENGTH OF THE BUILDING, THEN THE FOUNDATIONS SHALL BE STEPPED AS DIRECTED BY THE ENGINEER, (REFERENCE 5/S-06, TYPICAL STEPPED FOOTING).



S-04

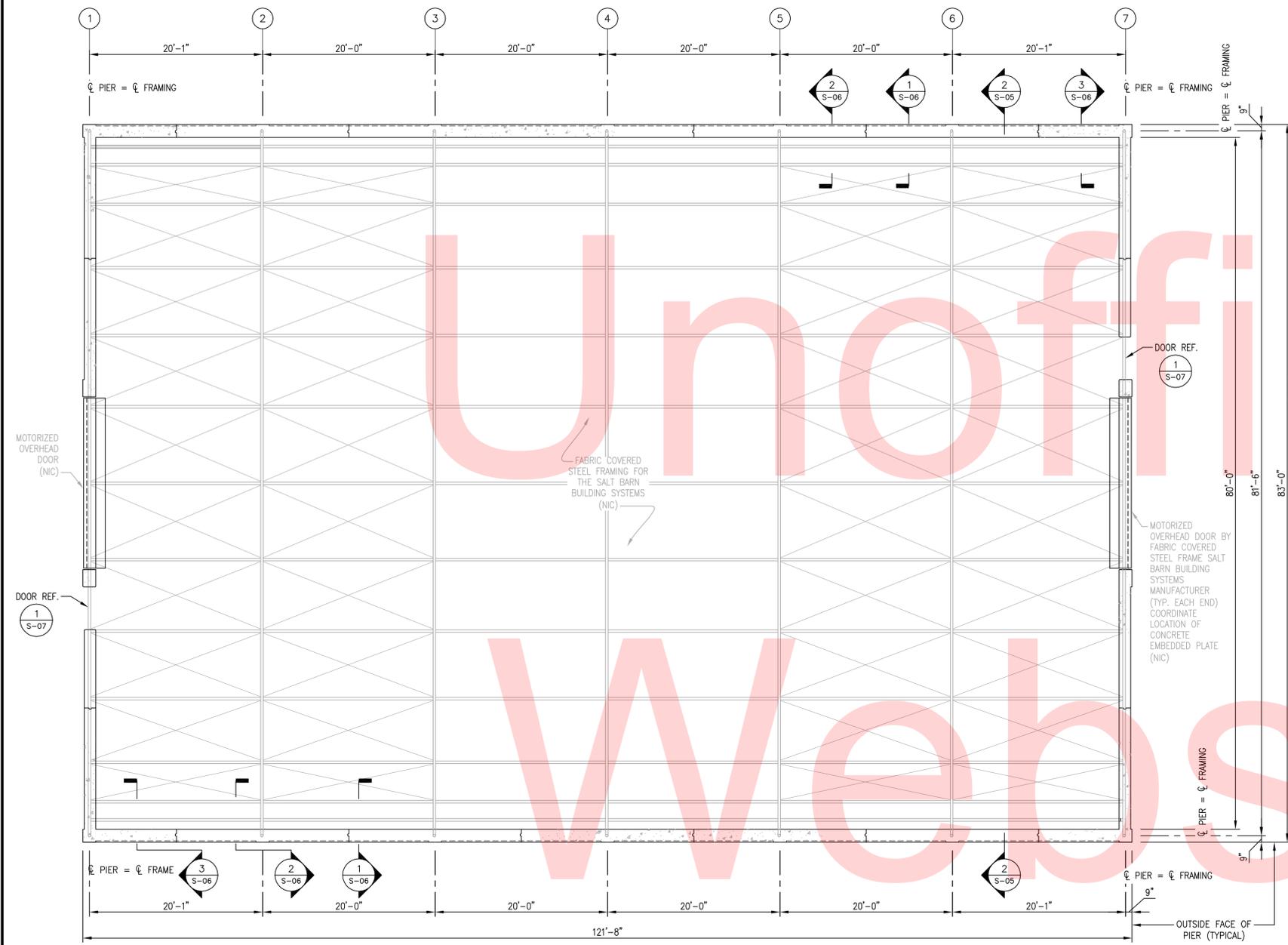
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ADDENDUMS / REVISIONS	

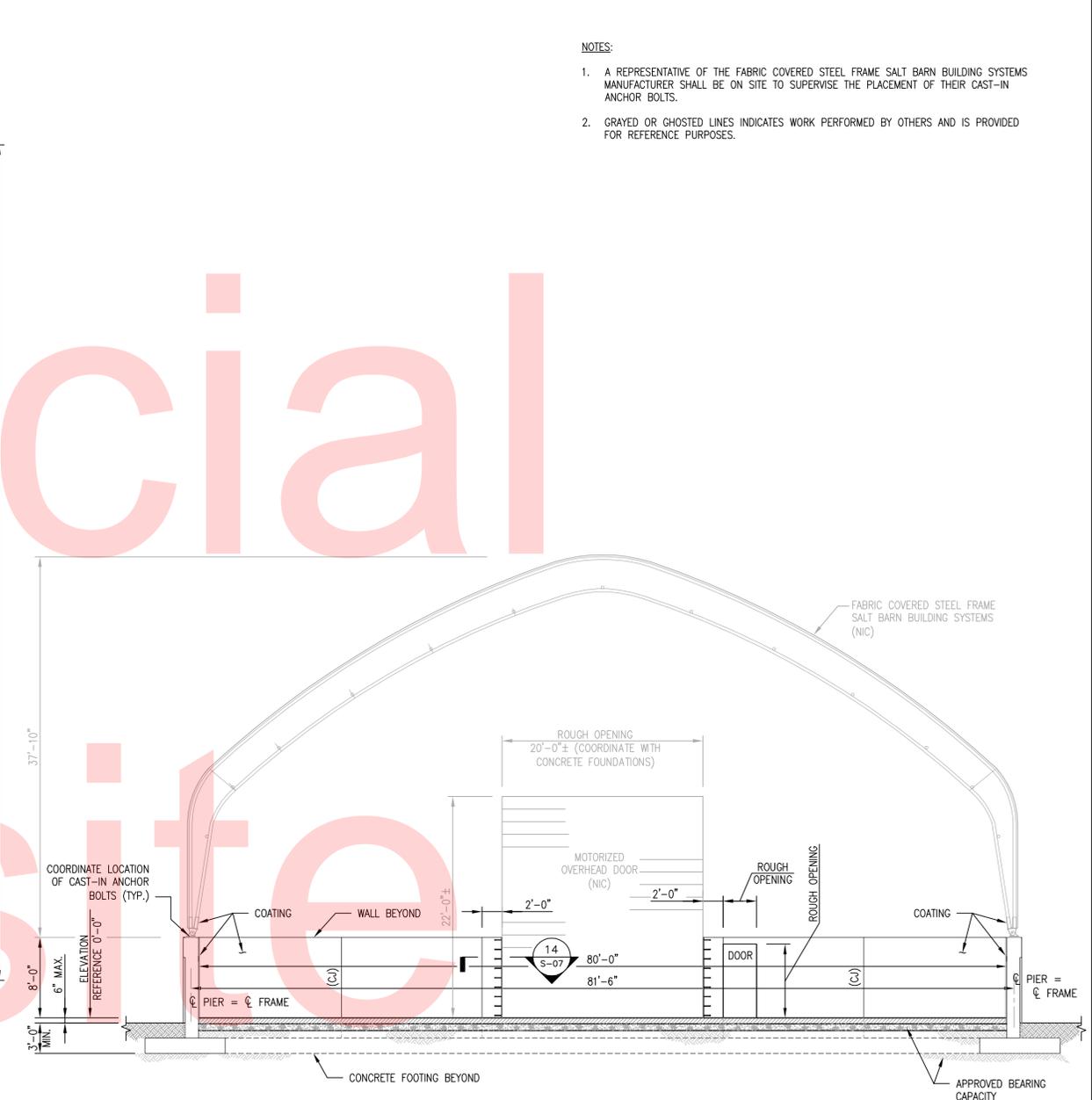
CONTRACT	DATE	03-18-2016
T201680107	DESIGNED BY:	NLL / JZ / DL
COUNTY	CHECKED BY:	JHB
STATEWIDE		

SHEET NO.	5
TOTAL SHTS.	8

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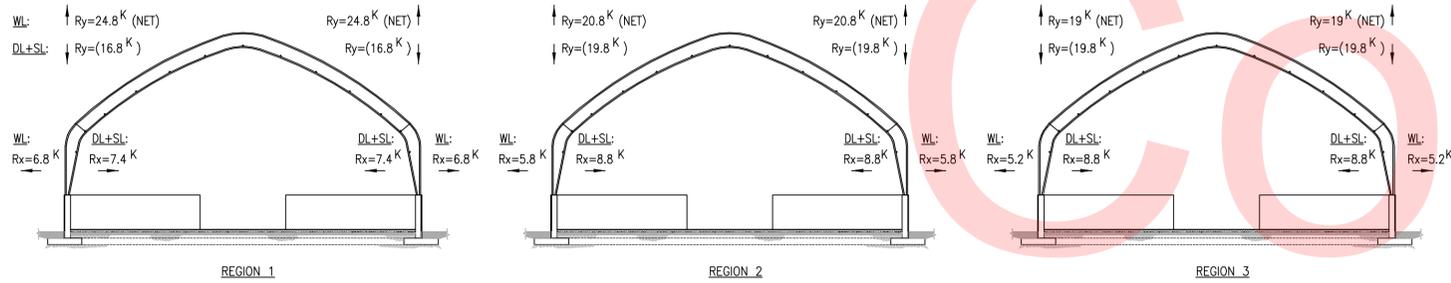


1 SALT BARN - FRAMING PLAN
SCALE: 1/8" = 1'-0"



2 SALT BARN - SECTION
SCALE: 1/8" = 1'-0"

WL: WIND LOAD
SL: SNOW LOAD
DL: DEAD LOAD



ASSUMED PRELIMINARY REACTIONS - 80x120
SCALE: N.T.S.

NOTES:



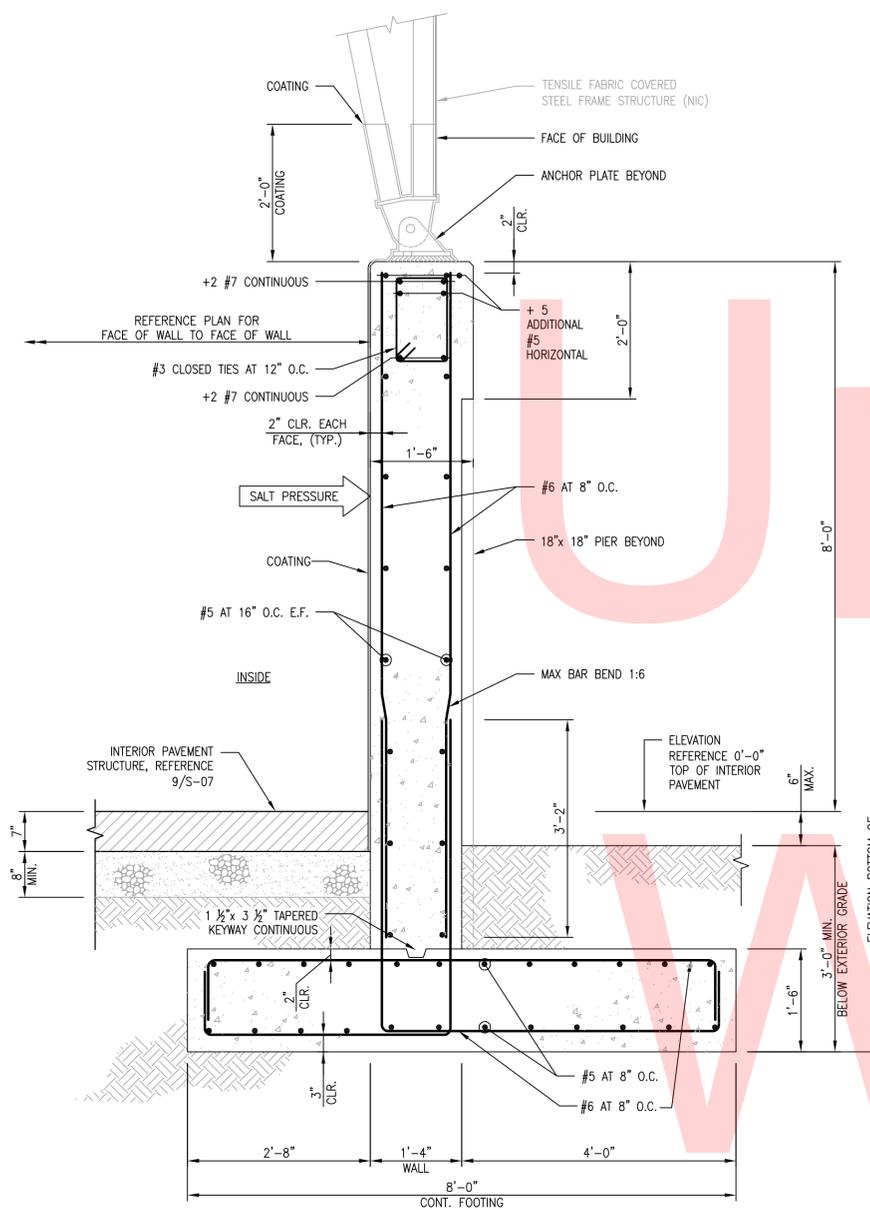
4' 0 4' 8'
SCALE: 1/8" = 1'-0"

S-05

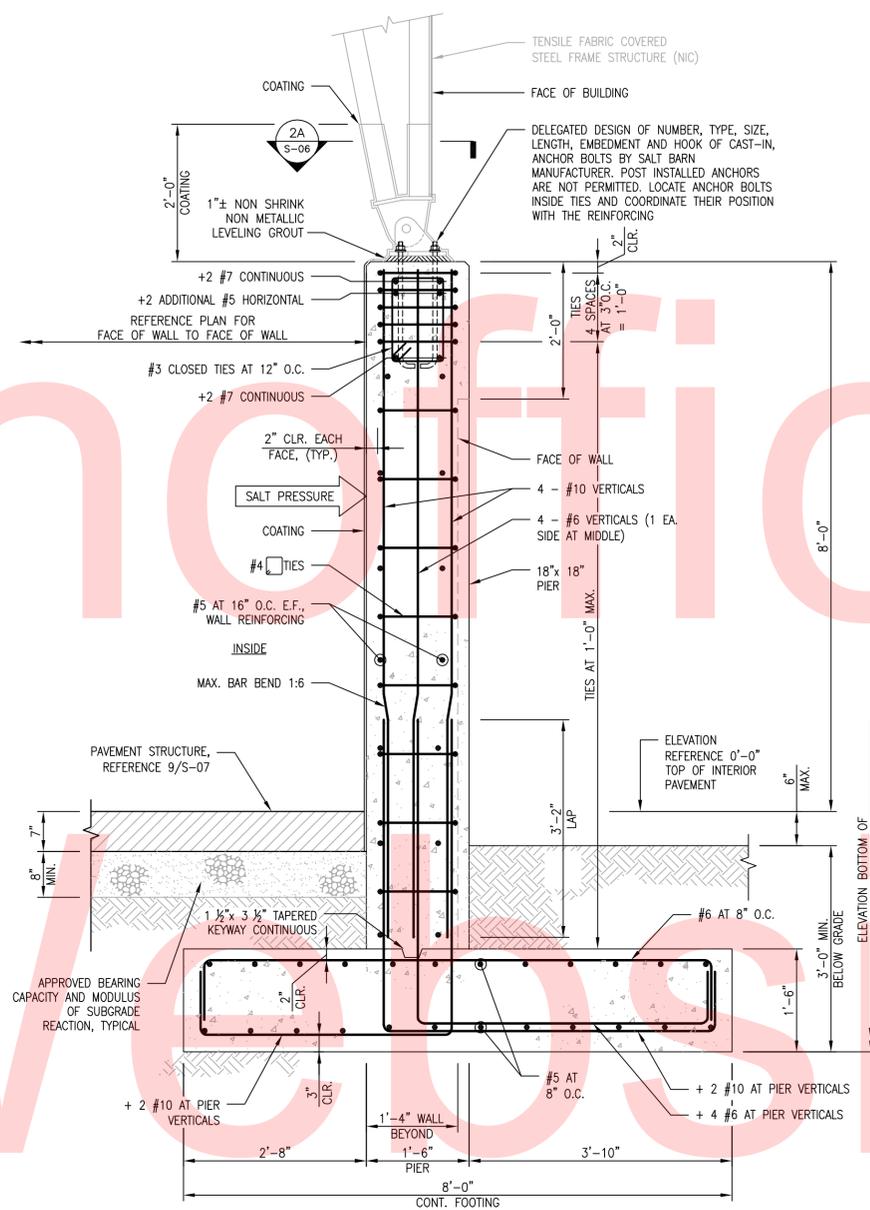
ADDENDUMS / REVISIONS	

CONTRACT	DATE	03-18-2016
T201680107	DESIGNED BY:	NLL / JZ / DL
COUNTY	CHECKED BY:	JHB
STATEWIDE		

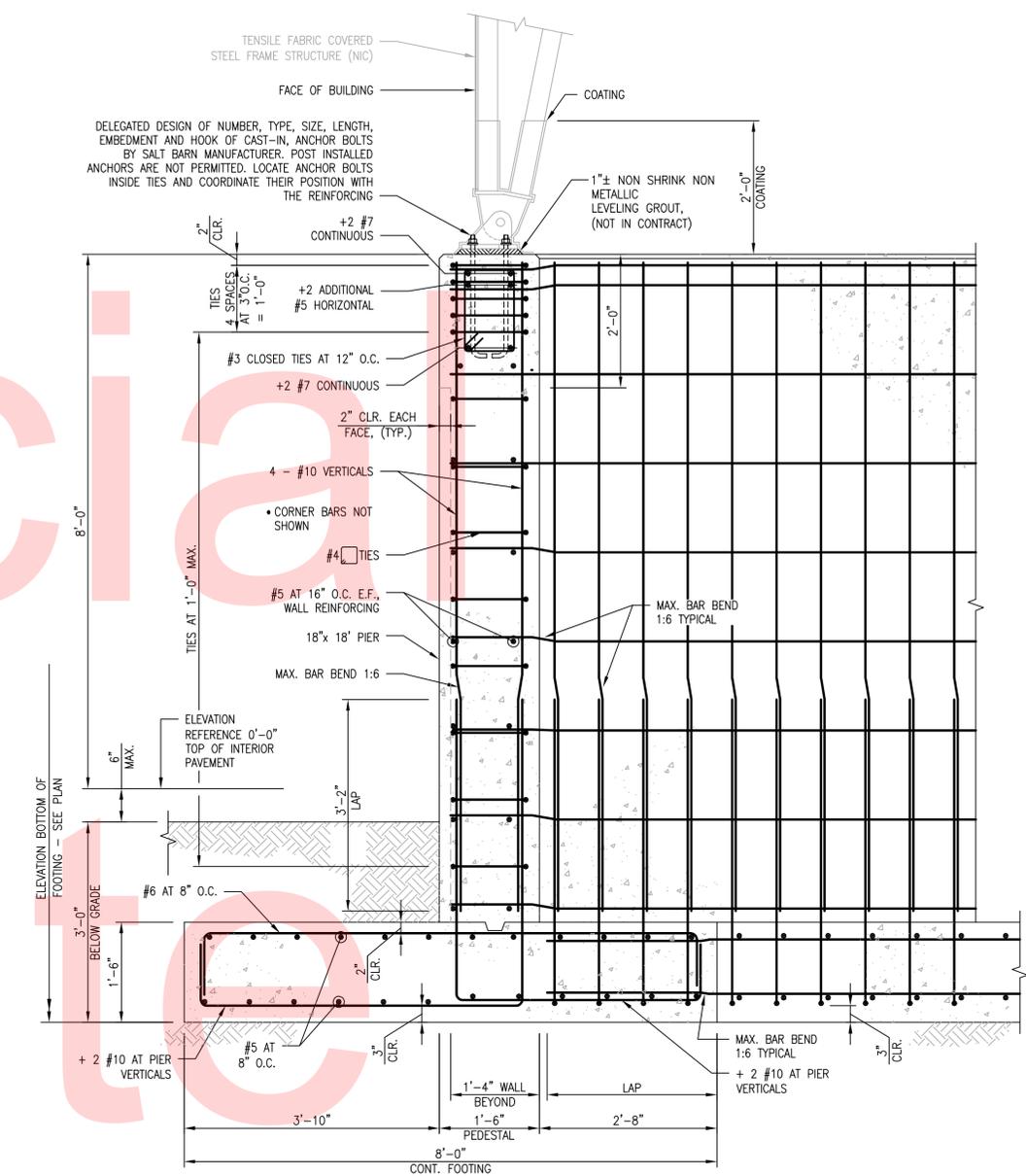
SHEET NO.	6
TOTAL SHTS.	8



1 CONCRETE WALL
SCALE: 3/4" = 1'-0"

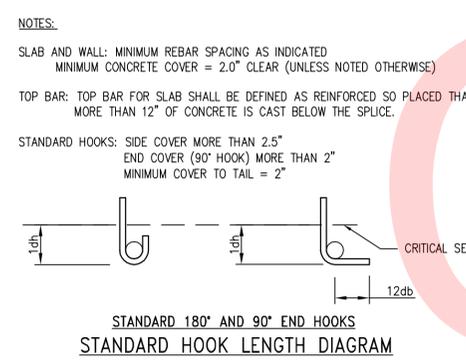


2 CONCRETE PIER AT INTERIOR
SCALE: 3/4" = 1'-0"

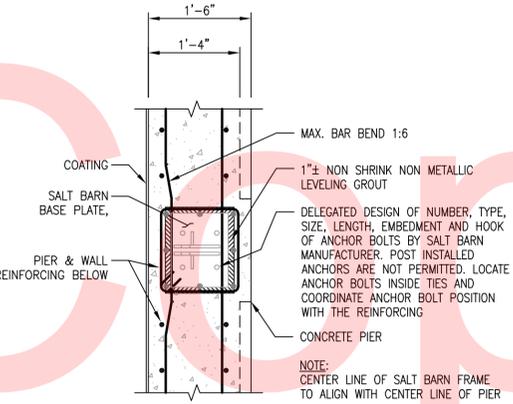


3 CONCRETE PIER AT CORNER
SCALE: 3/4" = 1'-0"

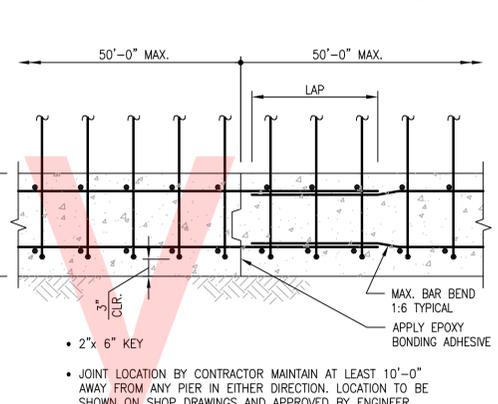
LAP SPLICE SCHEDULE (f'c = 4,000 PSI)						
BAR SIZE	SLAB AND WALL		STANDARD 90° HOOK		STANDARD 180° HOOK	
	OTHER BARS	TOP BARS	1dh	12db	1dh	4db
#3	15"	22"	8"	5"	8"	2"
#4	19"	29"	10"	6"	10"	2"
#5	24"	38"	12"	8"	12"	3"
#6	29"	43"	15"	9"	15"	3"
#7	42"	63"	17"	11"	17"	4"
#8	48"	72"	19"	12"	19"	4"
#9	54"	81"	22"	14"	22"	5"
#10	60"	89"	24"	16"	24"	5"



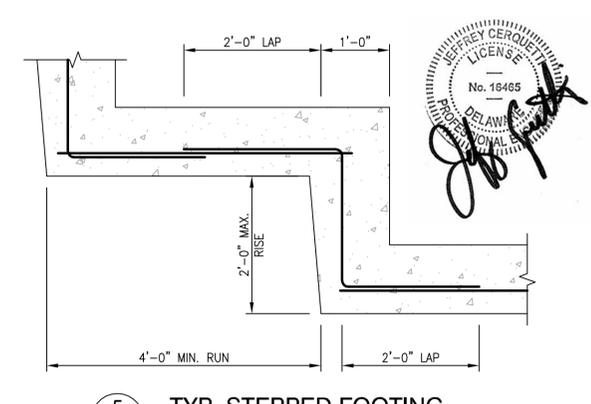
TYPICAL LAP SPLICE SCHEDULE & STANDARD HOOK LENGTH DIAGRAM



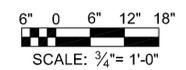
2A SALT BARN BASE PLATE
SCALE: 3/4" = 1'-0"



4 TYP. WALL FOOTING JOINT
SCALE: 3/4" = 1'-0"



5 TYP. STEPPED FOOTING
SCALE: N.T.S.



S-06

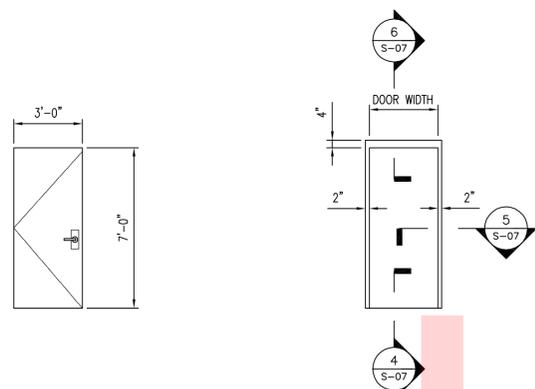


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ADDENDUMS / REVISIONS	

CONTRACT	T201680107	DATE	03-18-2016
COUNTY	STATEWIDE	DESIGNED BY:	NLL / JZ / DL
		CHECKED BY:	JHB

SHEET NO.	7
TOTAL SHTS.	8



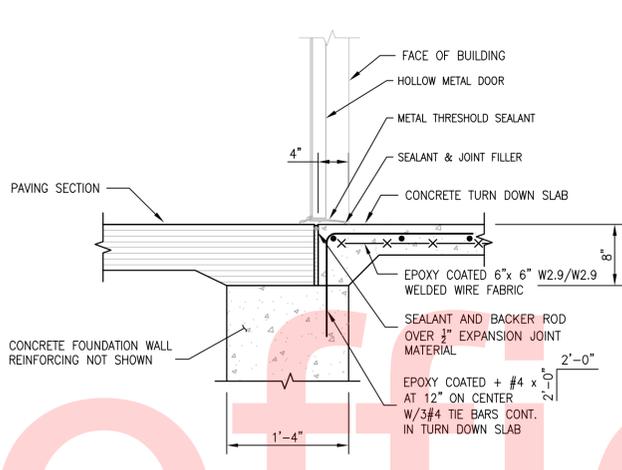
1 DOOR
S-07 SCALE: 1/4" = 1'-0"

2 FRAME
S-07 SCALE: 1/4" = 1'-0"

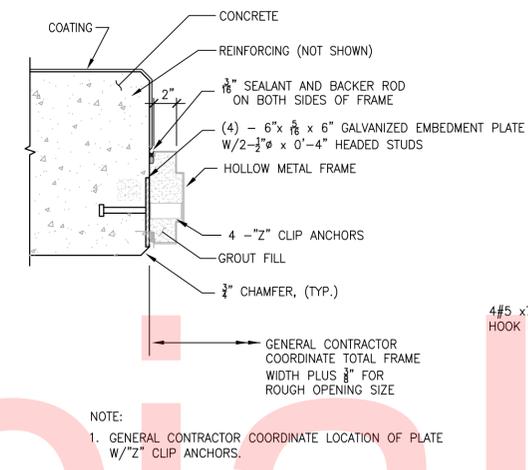
NOTES:

1. PAINT HOLLOW METAL DOOR AND FRAME. COLOR TO BE SELECTED BY OWNER. PROVIDE ZINC-RICH EPOXY PRIMER, EPOXY INTERMEDIATE COAT AND EPOXY TOP COAT FOR ALL MAIN DOORS AND FRAMES.
2. 1-3/4" THICK HOLLOW METAL DOOR.
3. STAINLESS STEEL HARDWARE SELECTED BY OWNER.

3 DOOR AND FRAME
S-07 SCALE: 1/4" = 1'-0"

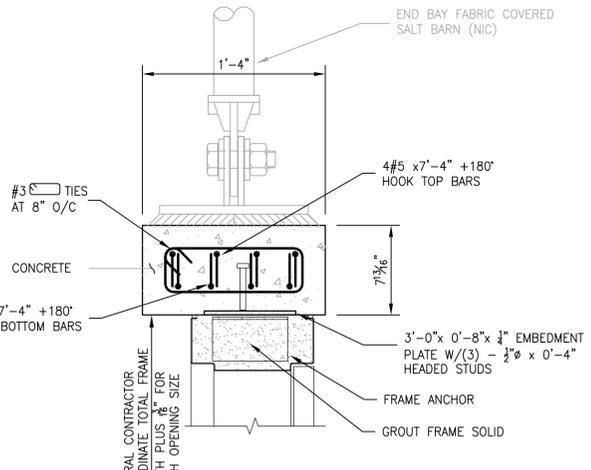


4 EXTERIOR DOOR SILL
S-07 SCALE: 1" = 1'-0"

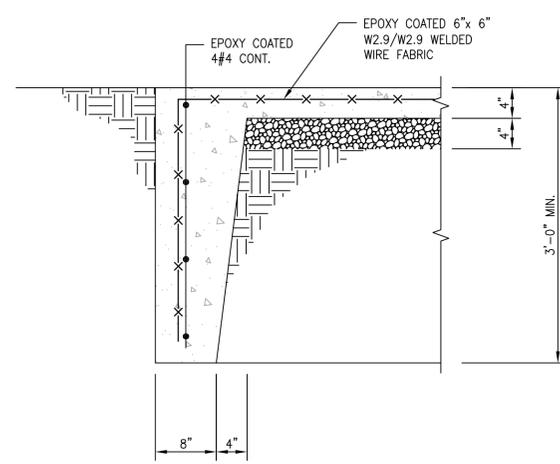


5 JAMB DETAIL
S-07 SCALE: 1-1/2" = 1'-0"

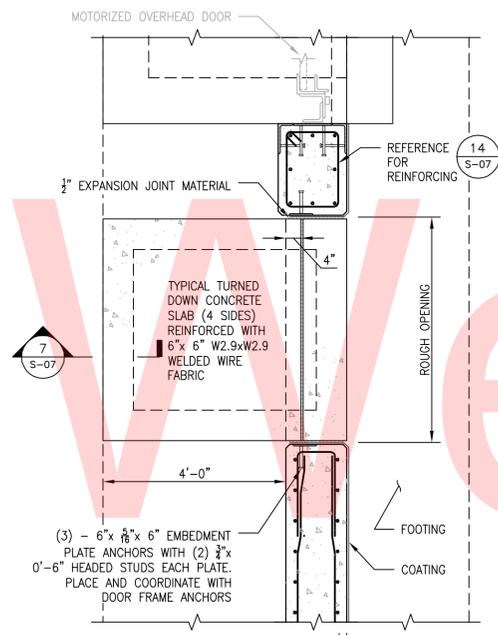
NOTE:
1. GENERAL CONTRACTOR COORDINATE LOCATION OF PLATE W/2" CLIP ANCHORS.



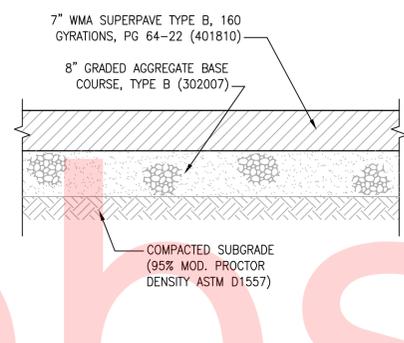
6 DOOR HEAD DETAIL
S-07 SCALE: 1-1/2" = 1'-0"



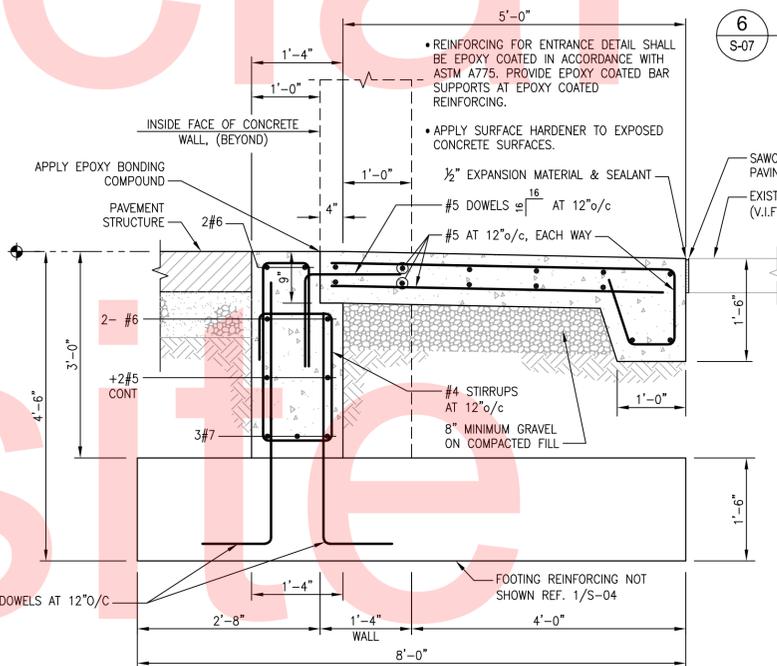
7 EXTERIOR TURN DOWN SLAB
S-07 SCALE: 1" = 1'-0"



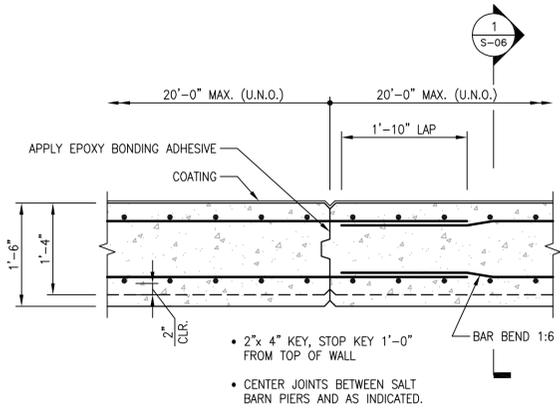
8 DOOR PAD
S-07 SCALE: 1/2" = 1'-0"



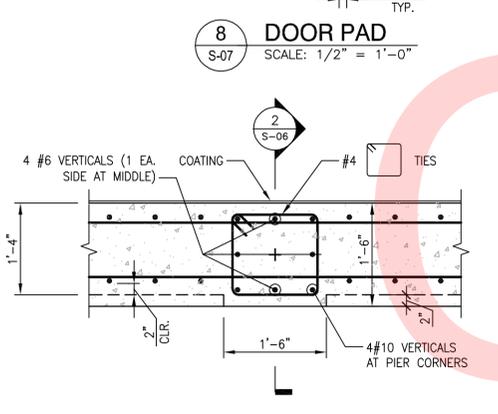
9 PAVING SECTION
S-07 SCALE: N.T.S.



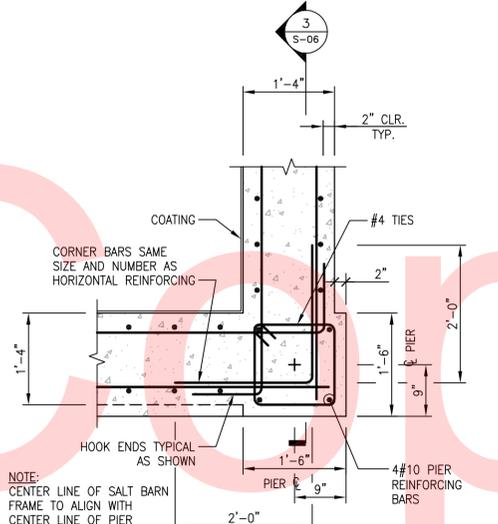
10 ENTRANCE DETAIL
S-07 SCALE: 3/4" = 1'-0"



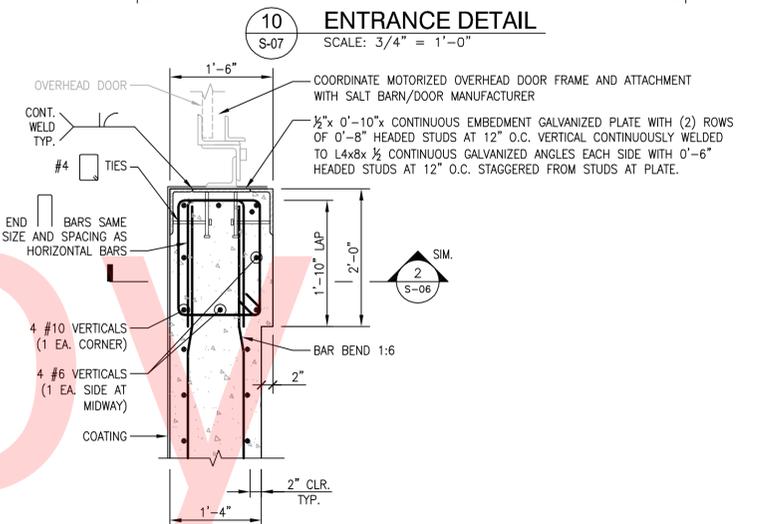
11 TYP. WALL CONSTRUCTION JOINT
S-07 SCALE: 3/4" = 1'-0"



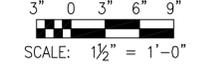
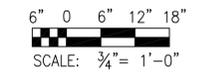
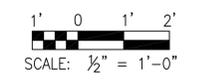
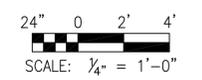
12 TYP. INTERIOR PIER
S-07 SCALE: 3/4" = 1'-0"



13 TYP. CORNER PIER
S-07 SCALE: 3/4" = 1'-0"



14 TYP. WALL END AT OVERHEAD DOOR
S-07 SCALE: 3/4" = 1'-0"



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ADDENDUMS / REVISIONS	

CONTRACT	T201680107	DATE	03-18-2016
COUNTY	STATEWIDE	DESIGNED BY:	NLL / JZ / DL
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